
HYDRAULIC DESIGN GUIDELINES



OFF-SYSTEM BRIDGE REPLACEMENT AND REHABILITATION PROGRAM

Front Cover:
Comite River
Off-System Bridge Replacement
East Baton Rouge Parish
Left – old timber pile bridge
Right – new concrete pile bridge

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PREFACE

The following is the Department's guidelines for the hydraulic design of off-system bridge replacements. This guideline and the "LA DOTD Hydraulics Manual" (located on LA DOTD's website) should be studied carefully prior to the design of any replacement structures.

The LA DOTD website is: www.dotd.louisiana.gov

Please direct any comments or questions to the Hydraulics Section at (225) 379-1306.

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SECTION 1 INTRODUCTION

1.1 Purpose

The Federal-Aid Off-System Bridge Replacement and Rehabilitation Program is, as the name implies, intended to replace structurally deficient bridges which are not part of the State Highway System. While the primary reason for replacing qualified structures is to remedy structural deficiencies, replacement structures will also be hydraulically designed according to applicable LA DOTD design criteria. As with all engineering designs, the object of the hydraulic evaluation is to determine the structure or structures which most economically meet the minimum criteria for the site. It is the goal of the FHWA and the LA DOTD to replace as many bridges as possible with funds allocated to this program. The hydraulic design is essential to that goal; a dollar saved at enough sites can be used to finance additional sites.

1.2 Hydraulic Design Criteria

Standard hydraulic design criteria and procedures as documented in the “LA DOTD Hydraulics Manual” will apply to all evaluations, except as amended in this guideline.

1.3 Computer Programs

Computer programs that are available for use in the hydraulic calculations in the Off-System Bridge Replacement Program are: the LA DOTD HYDR program series, (includes programs to compute peak discharge and size standard culverts), FHWA’s Water Surface Profile (WSPRO) modeling program, (to determine backwater and scour depths) and HEC-RAS.

One option for calculating the head losses and the hydraulic efficiency of a Precast 3 sided (P3S) structure is FHWA’s culvert analysis program, HY-8. The program has predefined shapes which assumes the bottom to be open. HY-8 also gives the user the option to define points for the shape of a P3S structure.

LA DOTD HYDR programs are available on the LA DOTD website under the “Pre-Construction – Road Design” area. The accompanying “HYDR User’s Manual” and the “LA DOTD Hydraulics Manual” are in a .PDF format on the website. They may also be purchased for a nominal fee through LA DOTD’s General Files Office located at the Headquarters Building in Baton Rouge, LA.

WSPRO and HY-8 may be obtained from the FHWA website:
<http://www.fhwa.dot.gov/engineering/hydraulics/software.cfm>.

1.4 Evaluation of Alternates

The hydraulics designer will evaluate all feasible alternate design structures for a site, and tabulate the hydraulic design data for each acceptable alternate.

All alternates are to be compared to the existing structure.

If an alternate is considered not feasible for any reason, a recommendation in this regard shall be included in the Hydraulics Report for the project site.

The final decision on the required replacement structure will be made after the plan-in-hand inspection.

Alternate design structures are discussed further in this guideline.

SECTION 2 ROADWAY GRADES

2.1 Finished Grade Elevation of Roadway

As a rule, grades will not be raised to prevent overtopping unless there is documented evidence that the Parish or Local Government has both the intention and the funds to raise the affected roadway. LA DOTD must have a written statement in the project file from the governing body of the parish stating their intention to raise the elevation of the existing roadway. A copy of said letter should be provided with the hydraulics report.

If the overtopping occurs only at the structure, however, with the roadway elevation on either side being high enough to prevent overtopping, investigations should be made into the feasibility of raising the finished grade of the structure above the 25-year stage. (This usually applies only when the length of the roadway to be raised is small.)

2.2 Elevation of the Proposed Bridge Structure Alternate

Generally, finished grade elevations of proposed bridge structures will match the elevations of existing structures.

Should the plan-in-hand party suggest that the elevation of the proposed bridge be raised or lowered, the reason for the recommendation must be documented.

At no time would the designer decrease the elevation of the low concrete on the deck below the DWSE (Design Water Surface Elevation) or have the proposed low chord lower than the existing low chord without being able to justify it to the governing parish body and the LA DOTD Chief Engineer.

2.3 Elevation of Roadway for Culvert Structure Alternates

The grade at a culvert replacement site will normally be the same as the existing grade.

SECTION 3 DESIGN STORM

3.1 Design Frequency and Discharge

The design criterion is the lesser of the 25-year flood or the overtopping discharge.

If the overtopping discharge is associated with a storm frequency less than a 5-year return interval, use the 5-year storm discharge to design the structure.

3.2 Overtopping Discharge

The overtopping discharge is that flow which will produce a headwater elevation equal to the low point of the adjacent roadway or bridge, whichever is lower. This headwater elevation is the normal water surface elevation plus the backwater created by the existing structure.

The overtopping stage elevation is considered to be the water surface elevation taken at a distance one bridge opening upstream from the upstream side of the embankment. (In WSPRO, this is the "Approach Constricted" section, located at a distance upstream from the bridge opening that is approximately equal to the sum of the bridge width and bridge length. See Section 4.4 of these guidelines for further details.)

3.3 Alternate Comparison

The existing structure and all alternate design structures should be analyzed for the same discharge for comparison purposes.

Table 3-1 is to assist in the selection of the appropriate drainage structure to design for at a particular site. This table is a general guideline only! Other conditions at the site may govern the selection of an alternate.

<u>STRUCTURE ALTERNATES</u>		
DESIGN DISCHARGE		STRUCTURE TYPE
cfs	(m³/s)	
Below 250	(< 7)	Pipe Only
250 – 750	(7 – 21)	Pipe or RCB
750 – 1000	(21 – 28)	Pipe, RCB, P3S or Bridge
1000 – 1600	(28 – 45)	P3S or Bridge
Above 1600	(> 45)	Bridge Only

Table 3-1

SECTION 4 BACKWATER

4.1 Alternates

Bridge hydraulics and culvert hydraulics are different in nature. The methods to compute the backwater or differential head are not the same for bridges and culverts. However, for economic comparisons, when choosing the size of alternates, the sizes should be based on the same area of opening, hydraulic equivalency, backwater limitations or allowable headwater elevation within the constraints of these guidelines.

4.2 Flood Zones

Neither a bridge nor a culvert should increase the headwater above the existing condition in a “Recognized Flood Zone”.

“Recognized Flood Zone” is a subjective term, but generally refers to flood sensitive areas with insurable buildings. An area where even a slight increase in backwater or headwater elevation would impact residential or other developed property would be an area where it would be unadvisable to increase the backwater or headwater elevation over the existing conditions. It should be documented in the report whether or not a site is in a flood sensitive area.

4.3 Allowable Headwater Elevation

In flatter terrain, where full-flow conditions exist, backwater will depend upon the “Flood Sensitivity” of the site and its proximity to developed property.

In more hilly terrain, the allowable headwater elevation may be governed by local buildings or the roadway elevation.

All alternate design structures must conform to the design allowable headwater elevation.

4.4 Backwater Determination

LA DOTD uses the design principles specified by FHWA in their publication HDS 01, “Hydraulics of Bridge Waterways” and their research report no. FHWA/RD-86/108, “Bridge Waterways Analysis Model: Research Report”. Both are available on the FHWA website.

The maximum backwater is considered to be produced by the water surface elevation taken at a distance one bridge opening upstream from the upstream side of the embankment. (In WSPRO, this is the “Approach” section, located upstream from the bridge opening at a distance approximately equivalent to the sum of the bridge width and bridge length.)

The quantity for the maximum backwater is the difference between the “Approach Constricted” and the “Approach Unconstricted” water surface elevations. That is, the water surface elevation

at the approach caused by the bridge in place minus the water surface elevation at the approach without the bridge. (In WSPRO this is automatically done when a bridge card is included. It is not necessary to run the program again without the bridge card.)

4.5 Allowable Backwater Ranges

Table 4-1 lists the allowed backwater range for the proposed structure for different site specific situations.

BACKWATER RANGES		
	<u>SITE LOCATION</u>	<u>BACKWATER RANGE</u>
A.	Where the overtopping discharge governs	0.1' – 0.5' (0.030 m – 0.150 m)
B.	Developed areas or areas subject to development	0.5' – 1.0' (0.150 m – 0.300 m)
C.	Remote area, development not likely within 20 years	1.0' – 1.5' (0.300 m – 0.450 m)

Table 4-1

SECTION 5 ALTERNATE STRUCTURES

5.1 Area of Opening

Should the area of opening of the proposed structure below the Normal Water Surface Elevation (NWSE) be less than that provided by the existing structure, information must be provided in the hydraulics report that will justify this design to the governing parish body and the LA DOTD Chief Engineer.

5.2 Flow Line

Culverts are to be placed at approximately the same slope as the natural stream. In case of headroom problems, they may be buried up to 1 ft (0.300 m) maximum. For complete guidelines, see the "LA DOTD Hydraulics Manual".

5.3 Structures to Be Evaluated

The following alternate design structures should be compared in deciding which alternate will replace the existing structure.

- a. Cross Drain Pipe or Cross Drain Pipe Arch (CDP or CDPA)
- b. Reinforced Concrete Box (RCB)
- c. Precast 3 Sided Structure (P3S)
- d. Bridge

A maximum of 4 lines of pipe or a 4-barrel Reinforced Concrete Box should be used.

For designing any other structure, contact the LA DOTD Hydraulics Section Administrator.

5.4 Cross Drain Pipe (CDP)

By definition, a Cross Drain Pipe contains a concrete/plastic and metal pipe option. Plastic pipes are the same size as concrete pipes, whereas metal pipes are at least one size larger in order to achieve the same hydraulic performance.

Pipes smaller than 60" (1500 mm) should have their metal alternates sized up 6" (150 mm) and pipes 60" (1500 mm) and above should have their metal alternates sized up 12" (300 mm).

A Cross Drain Pipe item must have the same number of lines for each pipe material, even though the sizes for the metal and concrete/plastic alternates will be different.

5.5 Cross Drain Pipe Arch (CDPA)

In cases of limited headroom, a Cross Drain Pipe Arch may be used. Plastic pipe is not commercially available in the arch pipe; therefore, a Cross Drain Pipe Arch item contains only concrete and metal pipe options.

A Cross Drain Pipe Arch is specified by its round size equivalence. Metal pipes are at least one size larger in order to achieve the same hydraulic performance. Pipes smaller than 60" (1500 mm) should have their metal alternates sized up 6" (150 mm) and pipes 60" (1500 mm) and above should have their metal alternates sized up 12" (300 mm).

A Cross Drain Pipe Arch item must have the same number of lines for each pipe material, even though the sizes for the metal and concrete alternates will be different.

5.6 Reinforced Concrete Box (RCB)

LA DOTD has standard plans for various sizes of box culverts. The standard plans for these RCB culverts are on the LA DOTD website.

5.7 Precast 3-Sided Structure (P3S)

Under certain conditions this may be the only viable option. In other cases it may be recommended based on physical features such as debris problems, etc.

For Off-System projects, a P3S structure will mean that it has a bottom slab (much like a RCB), unless otherwise decided by the plan-in-hand party.

The size of a Precast-3-Sided structure will be based on the site's conditions as well as the manufacturer's lengths and LA DOTD's directives. Since the manufacturer will not be known at the time of preliminary design, a general approximation to the size required should be used in accordance with LA DOTD guidelines.

LA DOTD requires span lengths not to exceed 32 ft (9.75 m).

Further restrictions and guidance are in Engineering Directives and Standards, EDSM No. II.3.1.5, "Use of Precast Reinforced Concrete 3 Sided Structures, Metal Arch Structures, and Structural Plate Structures". A copy may be found on the LA DOTD website.

5.8 Bridge

Bridges are discussed in Section 6.

5.9 Culvert Scour

Determination of scour and necessity of erosion protection for culverts is discussed in the "LA DOTD Hydraulics Manual".

SECTION 6 BRIDGE ALTERNATE

6.1 Bridge Type

Bridges will be designed as a slab span with 20 ft. (6 m) span lengths unless special conditions arise.

6.2 Abutment Slope

A minimum abutment slope of 3:1 for fill sections and cut sections will be used unless the LA DOTD Geotechnical Section has reviewed the site and suggested otherwise.

6.3 Area of Opening

To ensure that the WSPRO “design mode” analysis is a true reflection of a site, the actual area of opening underneath the bridge should be calculated and compared to the area given in the WSPRO output at the bridge section. The two areas should be within 10% of each other. If they are not, it may be necessary to input cross section cards at the bridge section of WSPRO, taking into consideration any improvements of the channel at the site, that is, any cut or fill to take place.

For the WSPRO analysis of the existing bridge, it may be more accurate to use the “fixed geometry” mode, thus eliminating the need for a hand calculation.

6.4 Bridge Scour

After the Plan-in-Hand inspection, if the bridge alternate has been chosen as the new structure, an estimate of the scour at the bridge site must be performed and sent as a supplement to the Hydraulic Report. The latest version of the FHWA Publication HEC 18 entitled “Evaluating Scour at Bridges,” will be used to determine the scour. This publication is available on FHWA’s website or may be obtained through the Hydraulics Section.

Some versions of WSPRO have incorporated the scour equations from HEC 18, and the contraction, pier and abutment scour can be calculated using the appropriate cards. Using this method or working directly with the equations from HEC 18 is acceptable. Engineering judgment should be applied in either case.

When the abutment slopes are going to be armored with flexible revetment or rip rap, an abutment scour analysis is not required. The total scour value would equal the contraction scour plus the pier scour.

A minimum total scour depth of 5 ft (1.500 m) is required at all bridge sites.

6.5 Erosion Protection

The designer will recommend any erosion control feature necessary. Where needed, the appropriate size rip rap should be specified.

A minimum 12 ft (3.7 m) bottom width is recommended when applying revetment or rip rap on the abutments. In other cases, the plan-in-hand party will discuss the option of putting revetment across the bottom of the channel providing there are no adverse affects to the environment or hydraulics of the stream.

SECTION 7 ADDITIONAL DESIGN CONSIDERATIONS

7.1 General

Besides the hydrology for a project site, other factors can be just as important when deciding the best alternate structure to construct. Following in this section are design factors that should also be taken into consideration when deciding which proposed structure to recommend.

7.2 Historical Flood Information

Any and all flooding that has occurred in the past should be presented in the report. This information may come from published data obtained from the USGS or US Army Corps of Engineers, parish or city drainage officials or local residents.

7.3 Interagency Coordination (Letters of Inquiry)

It is important to find out if any other agencies have work in progress or proposed that would affect the hydraulics of the stream for the project. "Letters of Inquiry" should be sent to the following agencies:

- a) US Army Corps of Engineers
- b) USGS (U. S. Dept. of the Interior, Geological Survey)
- c) NRSC (Natural Resources Conservation Service)

7.4 Potential for Watershed Development

Whether or not the watershed is in a residentially or commercially developed area or has the potential to be so in 20 years future should be identified. This information is used to determine the acceptable backwater range for the proposed structure.

7.5 Utility Conflicts

Utility conflicts at the site can prohibit the use of a particular alternate. Any such conflicts should therefore be identified.

7.6 Detours

The length of an alternate route the traveling public has available to use when the road of the project site is under construction should be measured.

When a detour road is necessary to construct, the detour grade should be lower than the main road to provide relief should a large storm event occur. If however, the detour grade is the same as the main road, the drainage structure for the detour has to be the same size as the drainage structure for the main road.

7.7 Average Daily Traffic Count

The ADT should be determined for both the current and future conditions. Also, the road classification should be identified.

7.8 Structure Alignment

The alignment of the stream with both the existing structure and proposed alternates needs to be identified. When designing for a RCB culvert alternate, the standard plan list of RCB culverts should be reviewed. Most, but not all, of the standard RCB culverts have crossing options of 90°, 75°, 60°, and 45°.

7.9 Existing Erosion Problems/Scour Potential

Any existing evidence of erosion or scour should be identified. Any potential for erosion or scour problems should also be identified.

7.10 Debris

Many of the state's streams have various size debris floating down their waters. This can cause maintenance and scour problems if it is obstructed by the structure and not easily able to pass through. Therefore, any debris problems evident at the site should be noted and photographs showing the amount and size should be included in the Hydraulic Report.

7.11 Design Criteria

All hydrological and hydraulic criteria and design assumptions need to be documented.

SECTION 8 HYDRAULIC REPORT

8.1 General Requirements

Thorough documentation of all design assumptions and design decisions is critical. All factors, especially judgmental factors, governing the selection of such design parameters as allowable backwater, allowable headwater elevation, permissible velocity, etc., must be documented in the hydraulic report. The selection of the limiting design parameters is determined by the designer. However, the basis for the selection of the limiting factors must be defensible by sound engineering principle.

A “Hydraulic Report” for each site is required and must be stamped by the Professional Engineer in charge.

Each report should be bound, typed, properly indexed and neatly arranged. The pages of the report should be numbered for referencing purposes. Included in each report should be all the calculations contributing to the design of the proposed hydraulic structures. Brief commentary should also be included describing the conditions of the site, justification for the proposed structure, and the effect the proposed structure will have at the site.

When multiple reports are submitted for review, each individual report should be bound separately from the other reports.

8.2 Comparison of Alternates

A table comparing the existing structure to all alternates and a Hydraulic Data Table for each bridge site are required in the report. Blank forms are included in Appendices A & B. The information contained on the tables should be based on the calculations in the report.

8.3 Outline of the Hydraulic Report

Below is an outline of the contents that should be included in each Hydraulic Report. Any other information not listed on the outline that is pertinent and would facilitate the review of the report should also be included.

- I. Summary Page
- II. General Project Information
 - A. Bridge Location
 - B. Existing Structure
 - C. Vicinity Map of the Bridge Site
 - D. Existing Bridge Site Plan
 - E. Existing Road Profile at Bridge Site

F. Photographs

- a) Color photographs of the existing bridge site should be in the report.
- b) The following views are to be included in the photographs:
 1. Downstream channel, taken from bridge
 2. Upstream channel, taken from bridge
 3. Typical view of the roadway, taken from bridge
 4. View of the opening provided by the structure

III. Design Considerations

- A. Historical Flood Information
- B. Interagency Coordination – Letters of Inquiry
- C. Potential for Watershed Development
- D. Utility Conflicts
- E. Detour Distance
- F. Average Daily Traffic Count
- G. Structure Alignment
- H. Existing Erosion Problems/Scour Potential
- I. Design Criteria

IV. Design Analysis

- A. Hydrology
 1. Drainage Area Map
 - a) Include a readable color copy of the USGS quadrangle map with the drainage area drawn out. The name of the quadrangle map, scale, and the points used in determining the basin slope should be indicated on the map.
 2. Peak Discharge Information & Calculations
 3. Discharge – Frequency Curve
- B. Hydraulics
 1. Cross Sections at Existing Bridge
 2. Stage – Discharge Curve
 3. Bridge Hydraulic Calculations (WSPRO Input & Output)
 - a) Existing Bridge
 - b) Proposed Bridge(s)
 4. Culvert Calculations of All Alternates
- C. Recommendation
 1. Comparison Table of Structural Alternates to Existing Structure (See Appendix A)
 2. Cross Section of Proposed Alternates
 3. Hydraulic Data Table of Existing Structure and Proposed Structures (See Appendix B)

V. CD

- A. WSPRO (or HEC-RAS) Input and Output for the Existing and Proposed Structures on a CD.

8.4 Scour Supplement

The “Scour Supplement” is required after the plan-in-hand once it has been decided that the replacement structure is going to be a bridge.

It is to include the following:

- A. Scour Calculations
- B. Scour Recommendations
- C. Updated Hydraulic Data Table with the appropriate scour information

8.5 Revisions

For most cases, revisions may be sent as a supplement to the original report with an explanation of what is being replaced and why.

8.6 Final Submittal

A CD containing the final revised hydraulic report addressing all comments is to be submitted with the Final Tracings. The report should be in .PDF format. The CD will be kept by the Hydraulics Section for reference if questions arise about the project at a later date.

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APPENDICES

Off-System Bridge Rehabilitation & Replacement Program

COMPARISON TABLE OF STRUCTURAL ALTERNATES TO EXISTING STRUCTURE

	EXISTING STRUCT.	BRIDGE ALT.	P3S ALT.	RCP / PP ALT.	CMP ALT.	RCB ALT.
SIZE, TYPE & NO.						
DESIGN FREQUENCY						
DESIGN DISCHARGE						
FINISHED GRADE ELEVATION						
LOW ROADWAY ELEVATION						
DESIGN WATER SURFACE ELEVATION *						
OUTLET VELOCITY						
DEPTH OF SCOUR – SOIL TYPE (Culverts Only)						
AREA OF OPENING						
DIFFERENTIAL HEAD						

* Includes the differential head

2005

Off-System Bridge Rehabilitation & Replacement Program Bridge Hydraulic Data Form

STATE PROJECT NO.:

STREAM NAME:

PARISH:

BEGINNING STATION:

STRUCTURE NO.:

FINISH GRADE ELEV. (ft):

PREPARED BY:

BRIDGE LENGTH (ft):

DATE:

LOW ROADWAY ELEVATION (ft):

NOTE: THE FOLLOWING HYDRAULIC DATA TABLE IS TO BE PLACED ON THE PLANS.

HYDRAULIC DATA TABLE

Drainage Area (mi ²):					
Basin Slope (ft/mi):				PREDICTED SCOUR	
Flood Frequency (years)	Design Year	Design Year	100	Flood Frequency (years)	
Discharge (ft ³ /s)				Discharge (ft ³ /s)	
Structure	Existing Bridge	Proposed Structure	Proposed Structure	Contraction Scour Depth (ft)	
Size & Type					
Design Water Surface Elev. (ft/MSL)				Maximum Local (Pier) Scour Depth (ft)	
Average Velocity (ft/s)					
Area of Opening (ft ²)				Abutment Scour Depth (ft)	
Backwater (ft)				Bridge Scour Elevation (ft)	

REMARKS:

SCOUR:

EXISTING BRIDGE:

DETOUR BRIDGE:

Off-System Bridge Rehabilitation & Replacement Program Bridge Hydraulic Data Form (Metric)

STATE PROJECT NO.:

STREAM NAME:

PARISH:

BEGINNING STATION:

STRUCTURE NO.:

FINISH GRADE ELEV. (m):

PREPARED BY:

BRIDGE LENGTH (m):

DATE:

LOW ROADWAY ELEVATION (m):

NOTE: THE FOLLOWING HYDRAULIC DATA TABLE IS TO BE PLACED ON THE PLANS.

HYDRAULIC DATA TABLE (METRIC)

Drainage Area (km ²):					
Basin Slope (m/km):				PREDICTED SCOUR	
Flood Frequency (years)	Design Year	Design Year	100	Flood Frequency (years)	
Discharge (m ³ /s)				Discharge (m ³ /s)	
Structure	Existing Bridge	Proposed Structure	Proposed Structure	Contraction Scour Depth (m)	
Size & Type					
Design Water Surface Elev. (m/MSL)				Maximum Local (Pier) Scour Depth (m)	
Average Velocity (m/s)					
Area of Opening (m ²)				Abutment Scour Depth (m)	
Backwater (m)				Bridge Scour Elevation (m)	

REMARKS:

SCOUR:

EXISTING BRIDGE:

DETOUR BRIDGE:

DISCHARGE/FREQUENCY GRAPH PAPER
ENGLISH & METRIC

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DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____

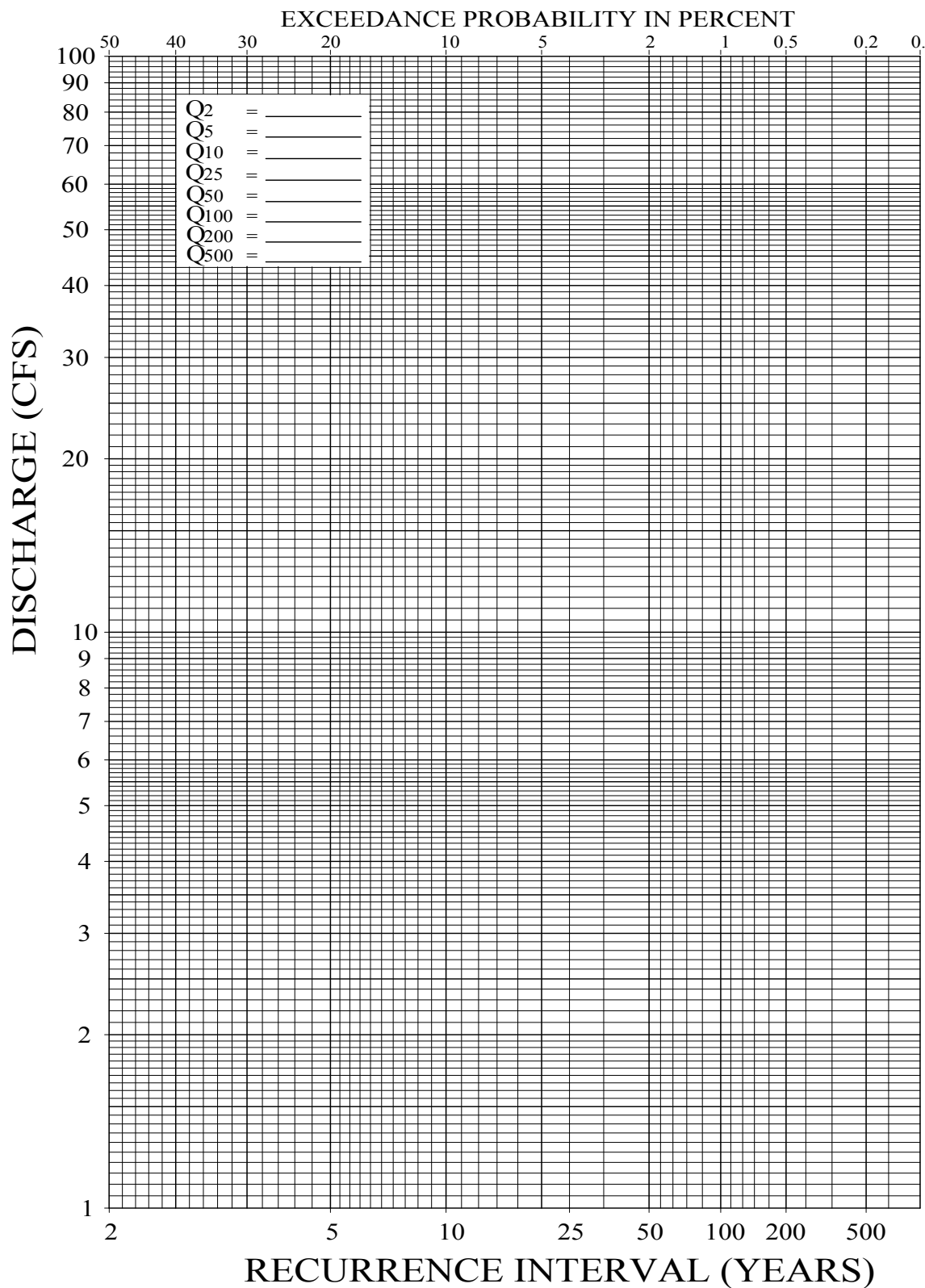
STREAM NAME _____

ROUTE _____

STRUCTURE NO. _____

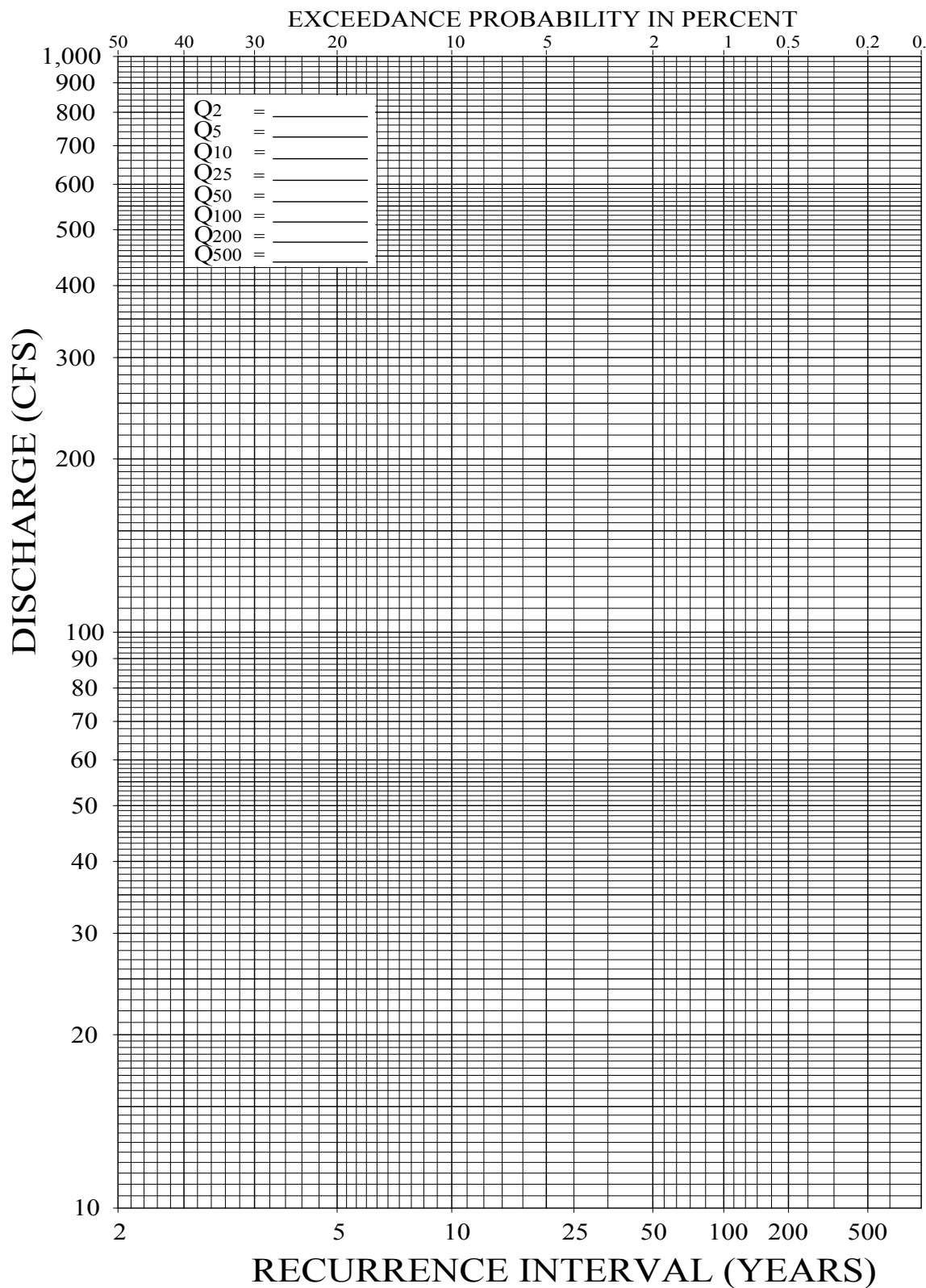
PARISH _____

BY: _____ DATE: _____



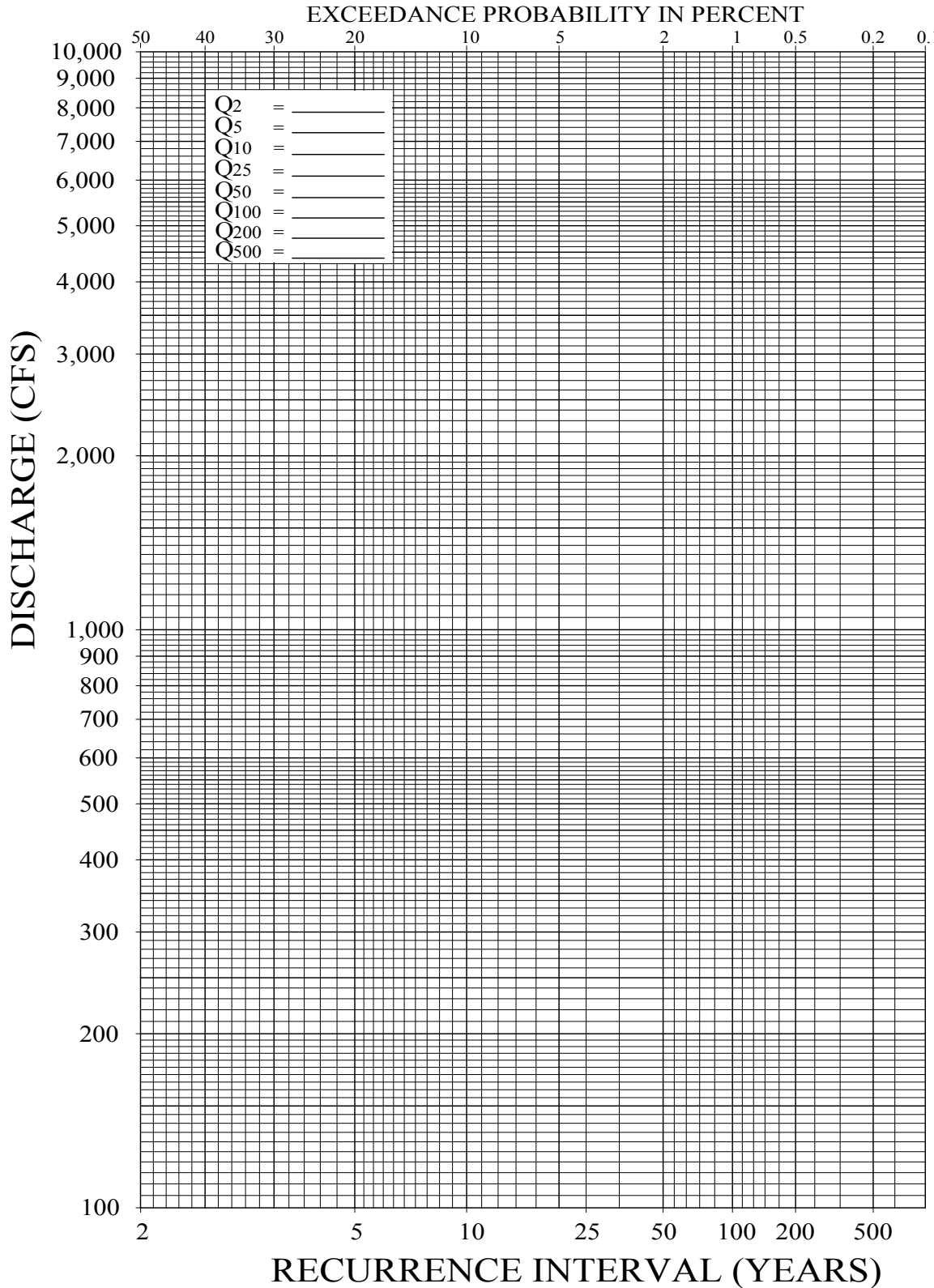
DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____ STREAM NAME _____
ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



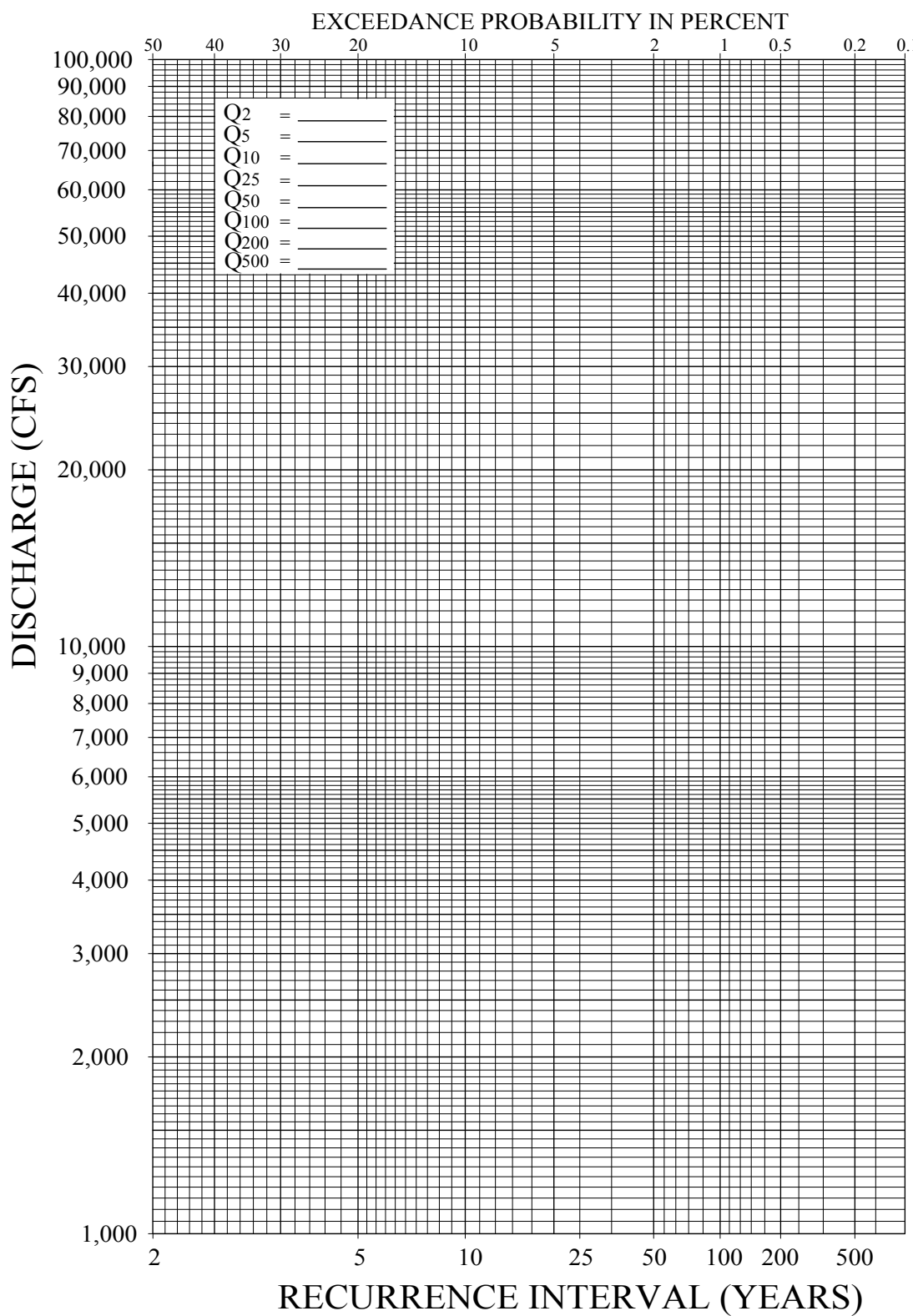
DISCHARGE - FREQUENCY CURVE

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ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



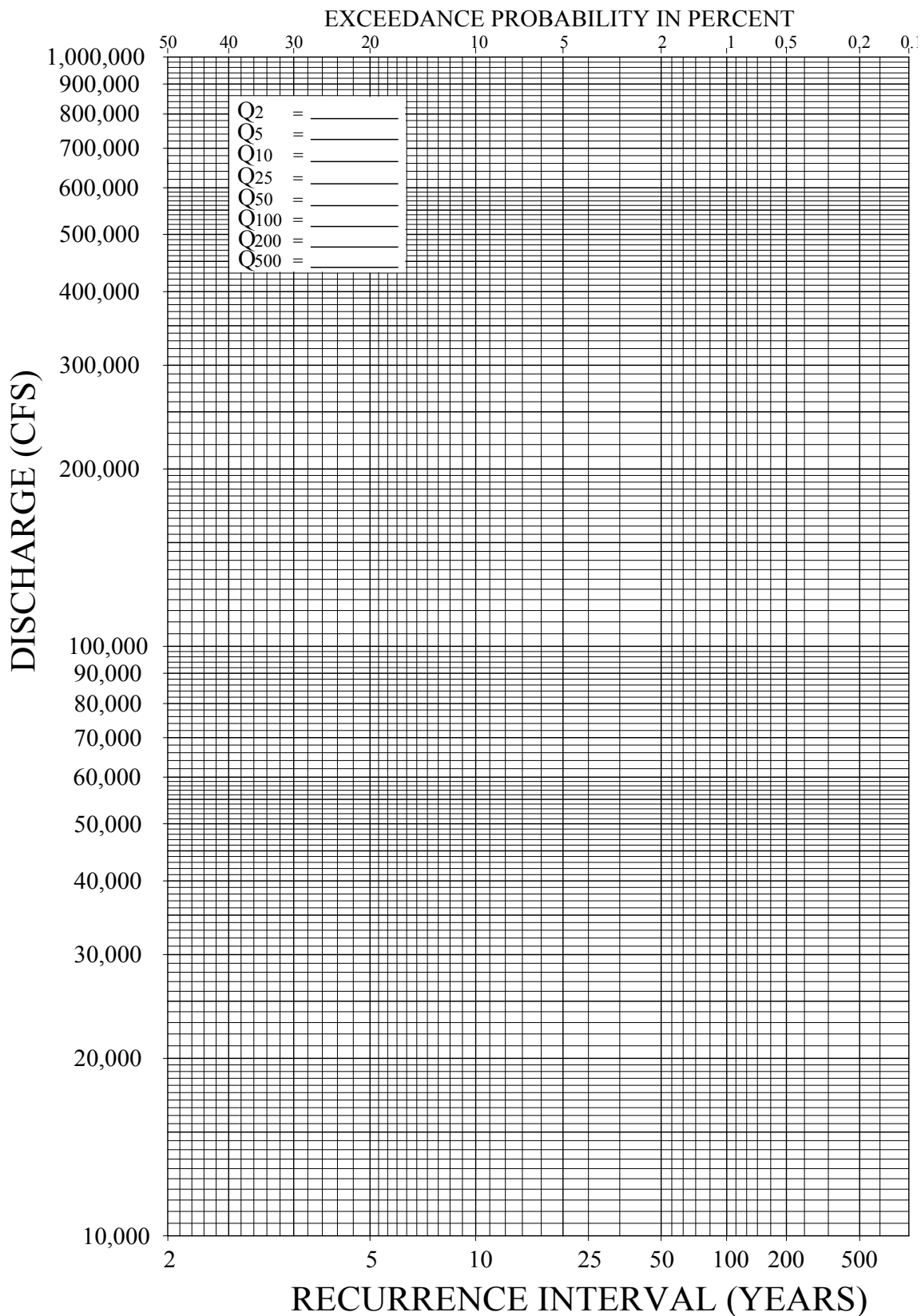
DISCHARGE - FREQUENCY CURVE

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ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



DISCHARGE - FREQUENCY CURVE

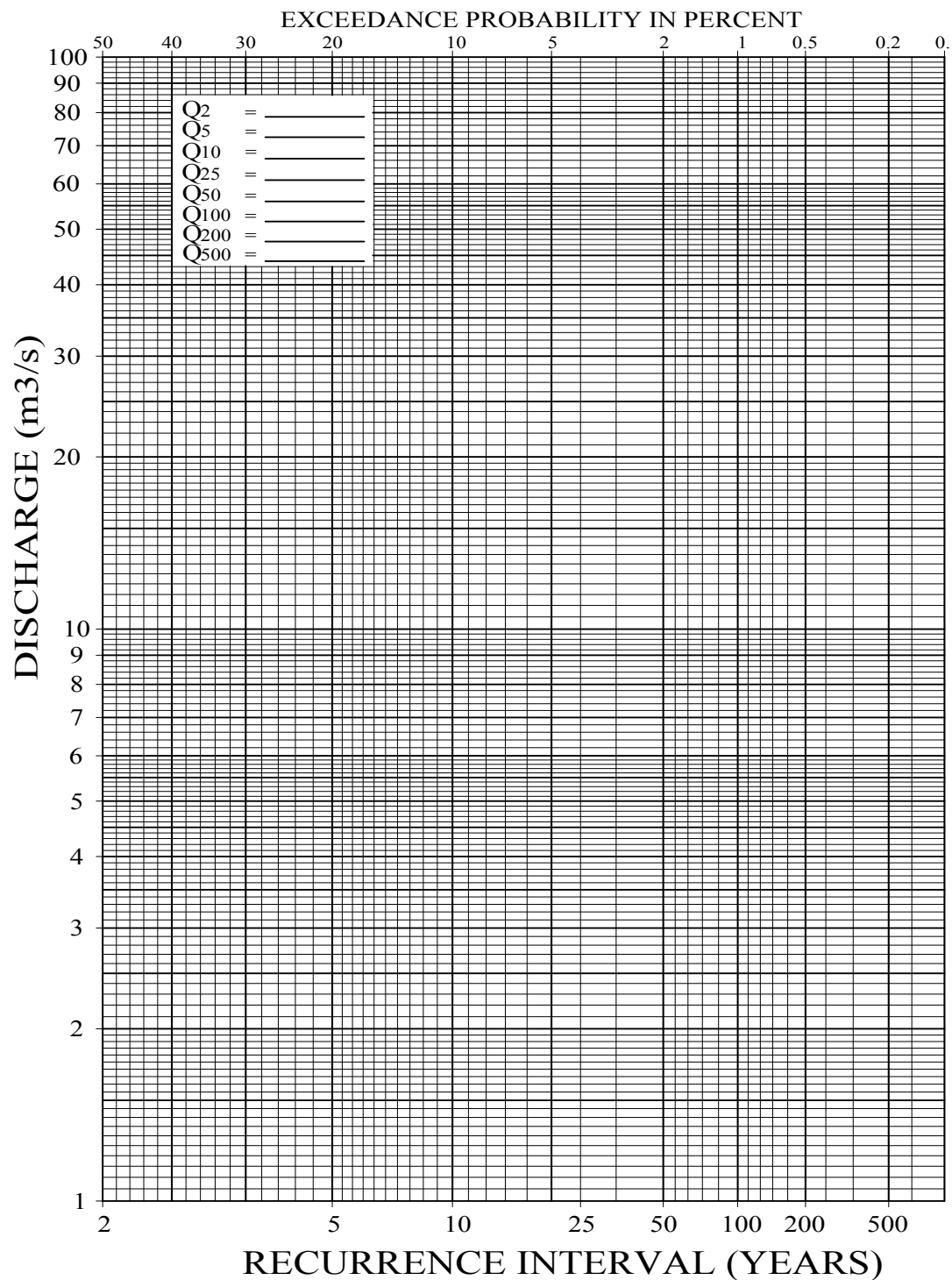
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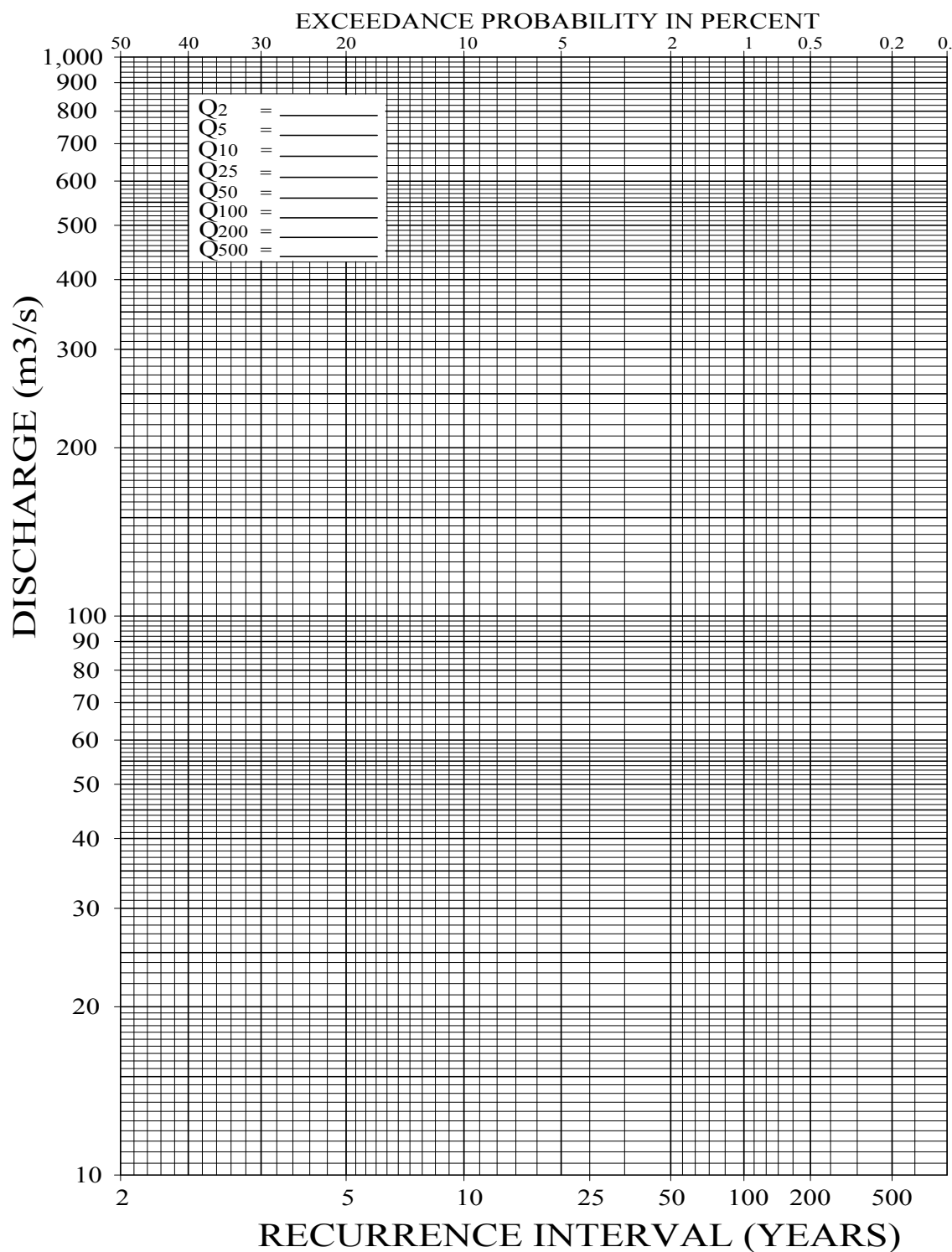
DISCHARGE - FREQUENCY CURVE

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PARISH _____ BY: _____ DATE: _____



DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____ STREAM NAME _____
ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____

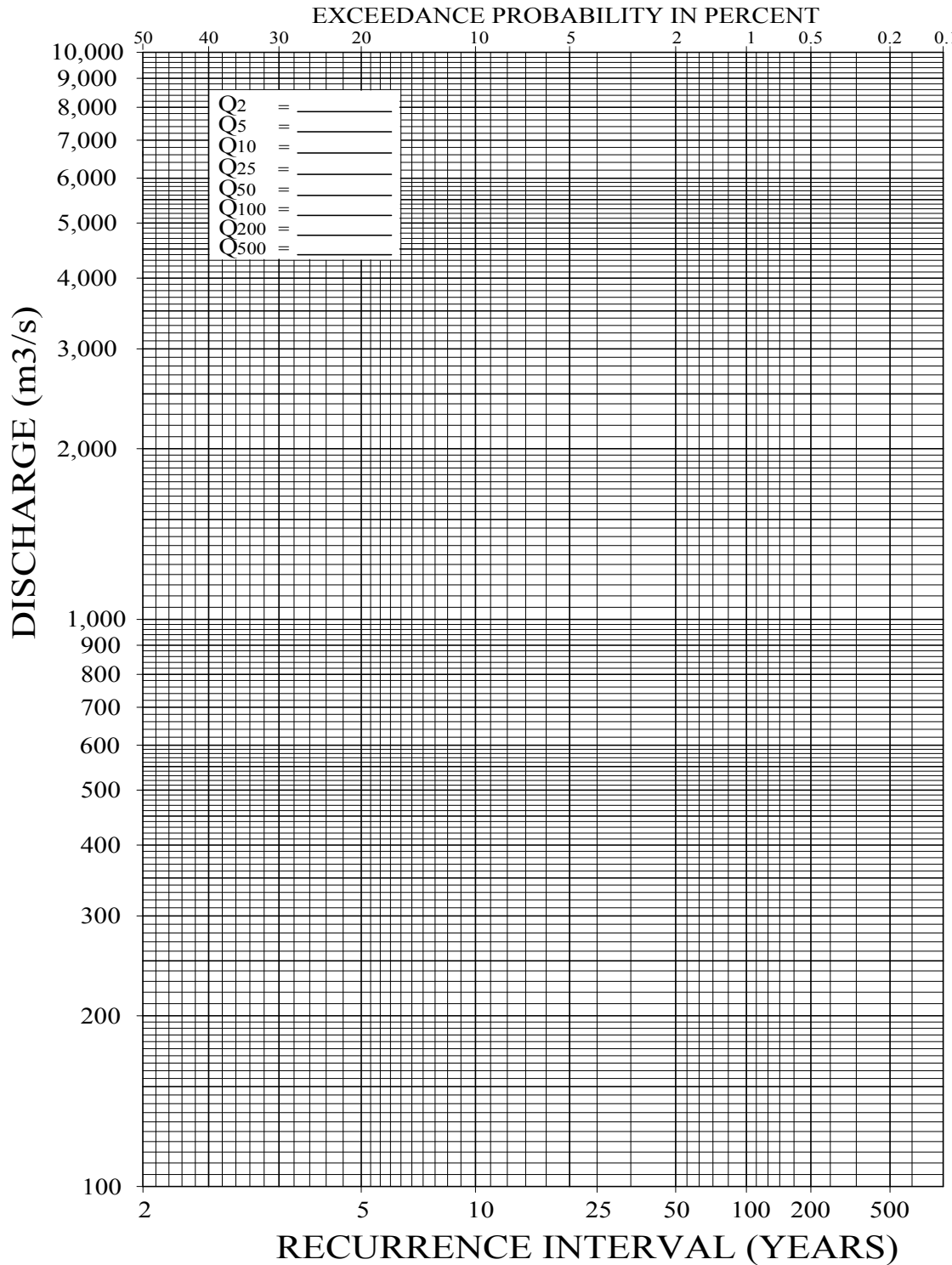
STREAM NAME _____

ROUTE _____

STRUCTURE NO. _____

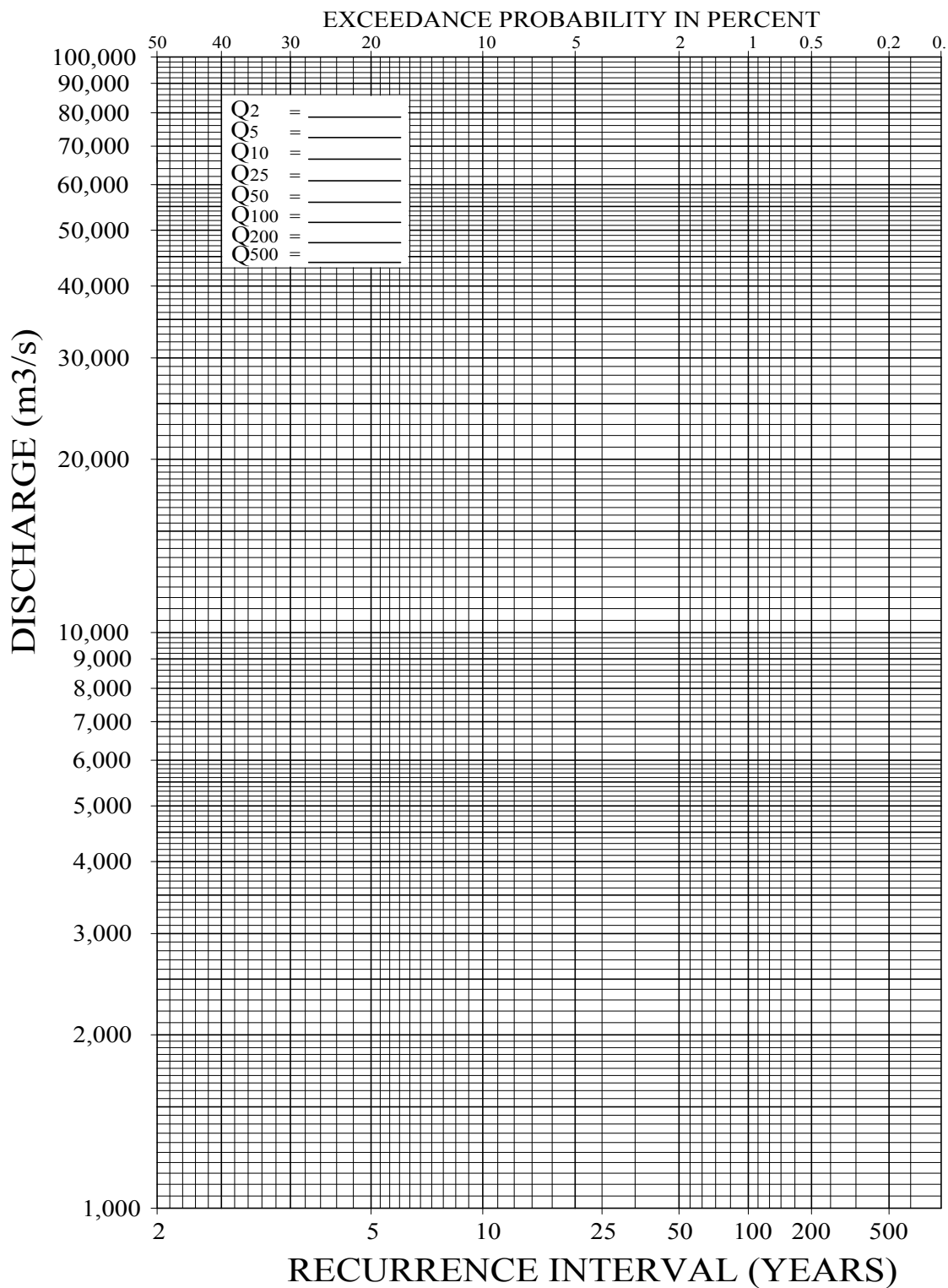
PARISH _____

BY: _____ DATE: _____



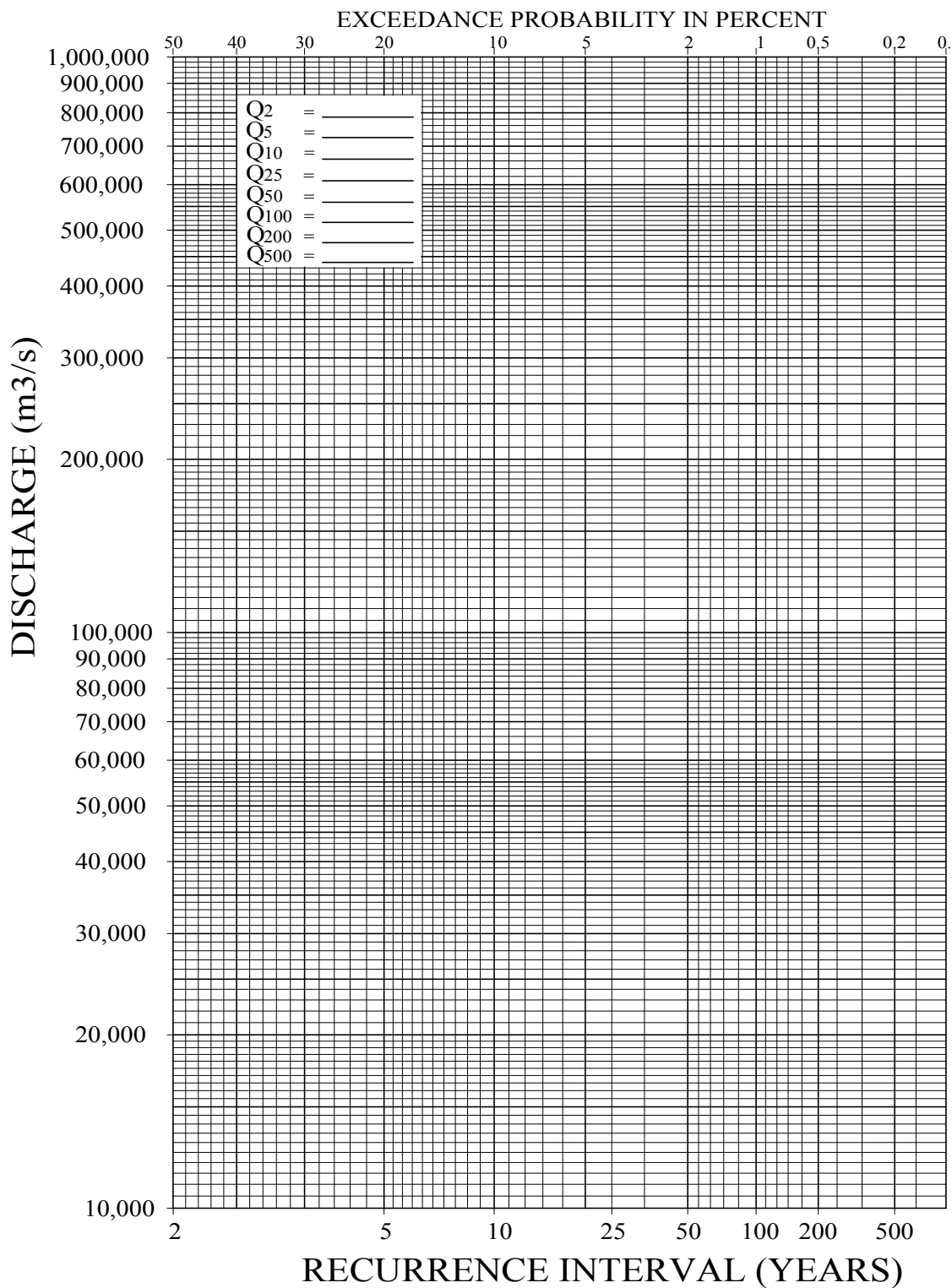
DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____ STREAM NAME _____
ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



DISCHARGE - FREQUENCY CURVE

STATE PROJECT _____ STREAM NAME _____
ROUTE _____ STRUCTURE NO. _____
PARISH _____ BY: _____ DATE: _____



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SAMPLE

HYDRAULIC REPORT

The following is a sample hydraulic report. The intent of providing the sample report is for general format only. **This sample report is not for setting policy.** (For policy see the first portion of the guidelines.) The methods, assumptions, and design criteria used in the calculations were considered appropriate for this particular bridge site. Each bridge site is unique and variations in the WSPRO models and culvert analyses may be appropriate and acceptable as long as they follow sound hydraulic reasoning.

Note: This report is a modification of an actual report submitted for review prior to this guideline publication.

For reproduction and cost considerations in publishing this manual, pages originally submitted on 11" × 17" paper have been reduced to an 8½ " × 11" sheet instead, and color photographs have been photocopied in black and white.

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HYDRAULIC REPORT

STATE PROJECT NO. 713-42-0000 (CONST.)
STATE PROJECT NO. 700-42-0000 (ENGR.)
F.A.P. NO. BRO-002S(000)
STRUCTURE NO. P42-32196-91474-1

LITTLE CREEK BRIDGE
ON TARVER ROAD

RICHLAND PARISH

PREPARED BY:

FIRM NAME

MAILING

ADDRESS

PHONE NUMBER:

DATE OF REPORT

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I. SUMMARY

LITTLE CREEK BRIDGE
STATE PROJECT NO. 713-42-0000
RICHLAND PARISH
STRUCTURE NO. P42-32196-91474-1

This report serves to document design investigations, computations, and decisions relative to the selection of a hydraulically suitable replacement for the existing 4-span, 52 ft long timber bridge on Tarver Road over Little Creek located at about 1.55 miles north of Mangham in Richland Parish. It is also intended to fulfill the requirements of the Federal Highway Administration FHPM 6-7-3-2, "Location and Hydraulic Design of Encroachments on Flood Plains."

The structural alternates considered to replace the existing 52 ft long timber bridge are:

One 80 ft (four 20 ft spans) Long Reinforced Concrete Bridge
With 3:1 Spill through Abutments
Proposed Finished Grade at Stream Crossing: 66.93 ft

or

Four 96" equiv. RCPA / 108" equiv. CMPA
Cross Drain Pipe Arch
Proposed Finished Grade at Stream Crossing: 66.93

or

Four 8 ft $\times$ 7 ft
Reinforced Concrete Box Culverts
Proposed Finished Grade at Stream Crossing: 66.93 ft

or

Two 28 ft $\times$ 7 ft
Precast 3 Sided Structure
Proposed Finished Grade at Stream Crossing: 66.93 ft.

Considering the project feasibility, area of opening, effective width of creek, discharge, and backwater created by the proposed structure for the design discharge of 1000 cfs for the 5-year frequency, the 4-span, 80 ft long concrete bridge at the existing grade is recommended.

LITTLE CREEK BRIDGE

STATE PROJECT NO. 713-42-0000
RICHLAND PARISH
STRUCTURE NO. P42-32196-91474-1

II. GENERAL PROJECT INFORMATION

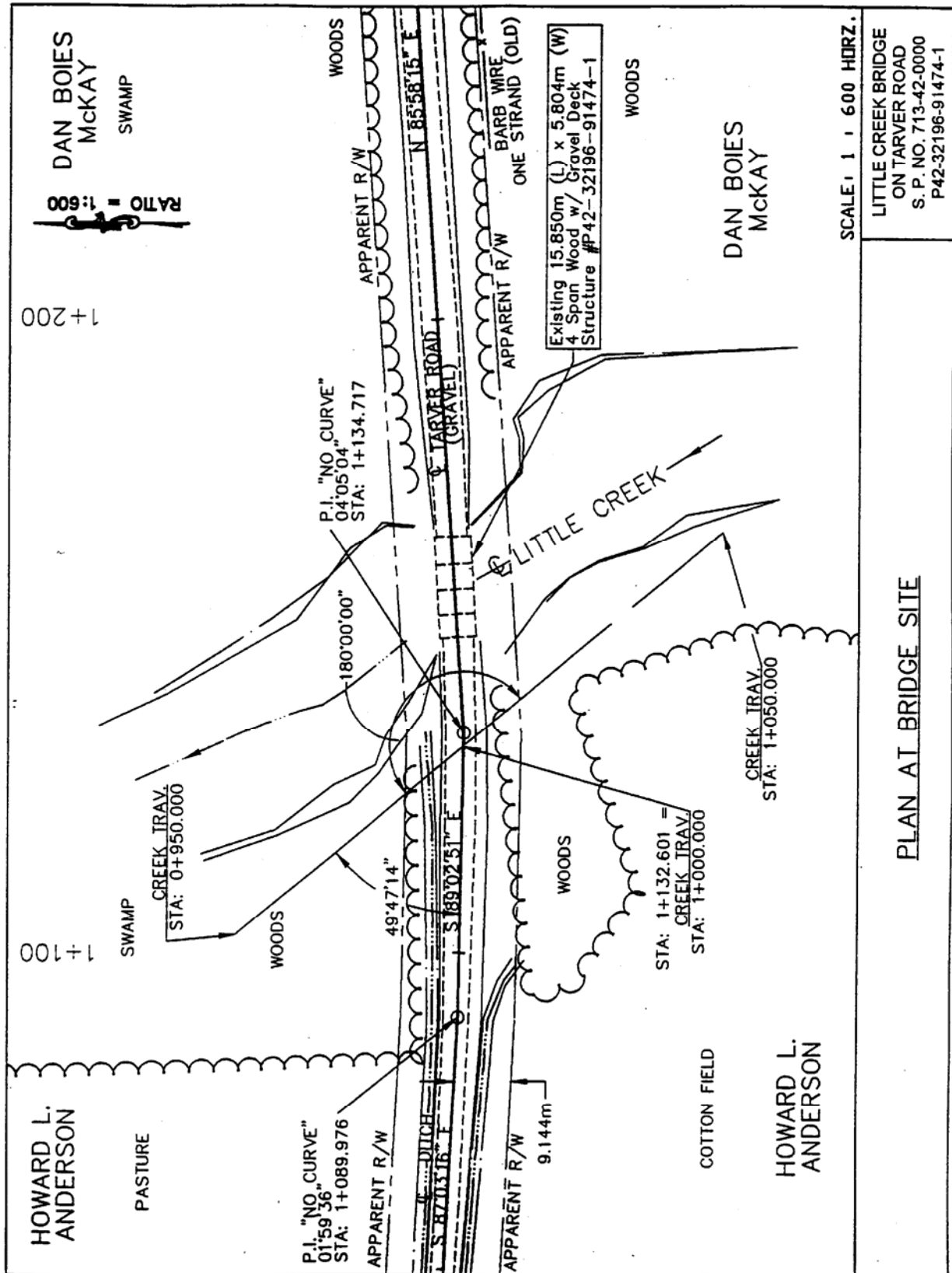
A. Bridge Location

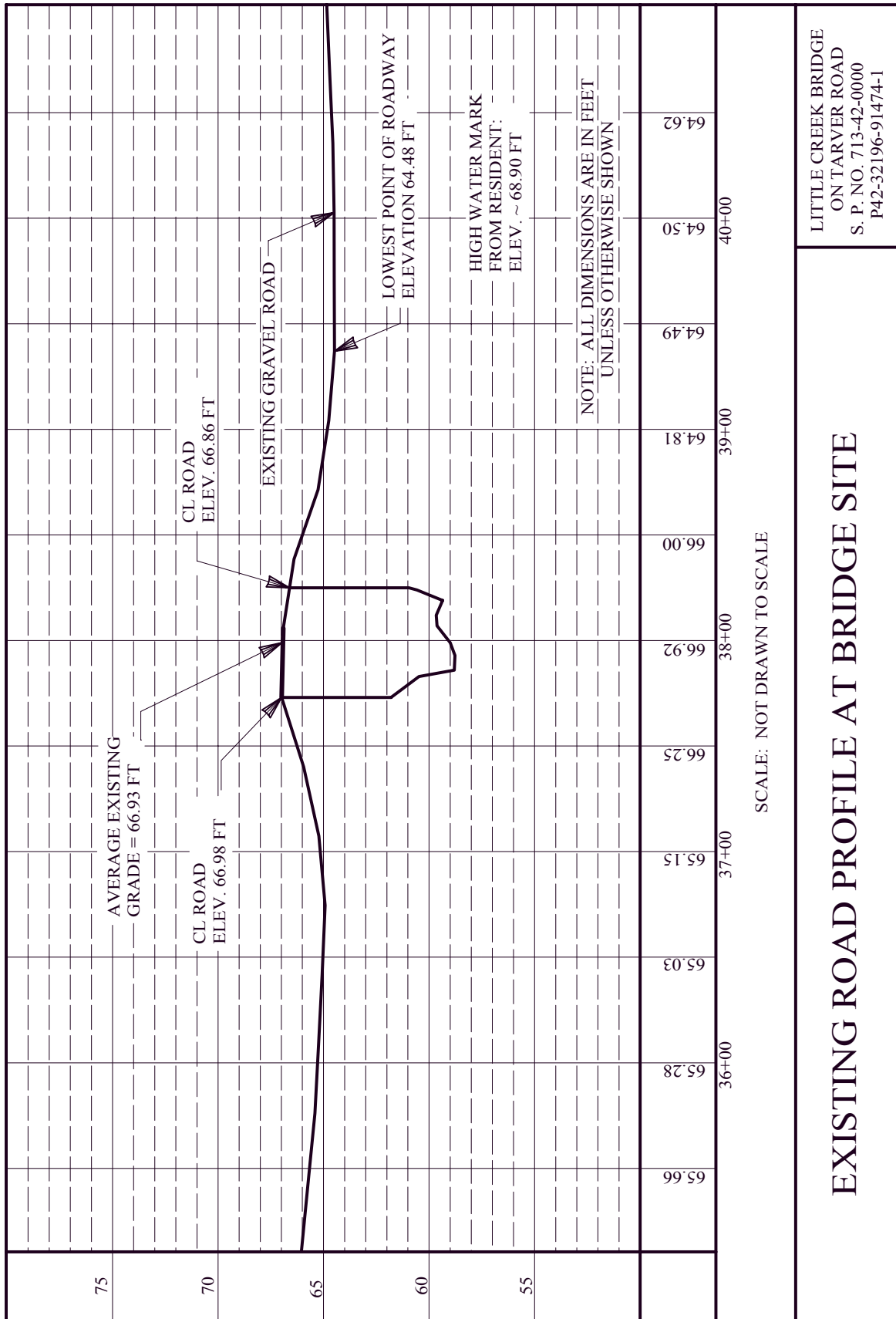
The subject bridge site is located about 1.55 miles north of Mangham in Richland Parish, where Tarver Road (a gravel road) crosses Little Creek at about a 60° angle. The location is more precisely shown on the vicinity map that follows.

B. Existing Structure

The existing structure is a 4-span, 52 ft long and 20 ft wide timber bridge. The structure is recommended for replacement by the Louisiana Department of Transportation and Development (LA DOTD). Accumulation of driftwood or scouring does not appear to be significant at this site. The existing bridge site plan and profile are shown in this section. The lowest point of the roadway is at 64.48 ft elevation.









PHOTOGRAPH 1: ROADWAY VIEW, LOOKING WEST FROM BRIDGE



PHOTOGRAPH 2: ROADWAY VIEW, LOOKING EAST FROM BRIDGE



PHOTOGRAPH 3: CHANNEL UPSTREAM, LOOKING SOUTH FROM BRIDGE



PHOTOGRAPH 4: CHANNEL DOWNSTREAM, LOOKING NORTH FROM BRIDGE



PHOTOGRAPH 5: BRIDGE UPSTREAM SIDE, LOOKING FROM CHANNEL BANK



PHOTOGRAPH 6: BRIDGE DOWNSTREAM SIDE, LOOKING FROM CHANNEL BANK

III. DESIGN CONSIDERATIONS

A. Historical Flood Information

No recorded data was found for the subject bridge by searching the U.S. Geological Survey and U.S. Army Corps of Engineers published stage-discharge records. Local residents have indicated that the high water mark is about 69 feet. The bridge is inside the 100-year flood hazard area (zone A) according to the “Flood Hazard Boundary Map” published by the Federal Insurance Administration of the Department of Housing and Urban Development.

B. Interagency Coordination – Letters of Inquiry

In order to determine if any planned or proposed improvements by other agencies will affect this site, letters of inquiry were sent to the U.S. Army Corps of Engineers, Natural Resource Conservation Service (NRCS), and U.S. Geological Survey (USGS). These letters requested that they notify us of any potential work that may have impact on this bridge replacement project. Copies of these letters and the responses received are included in Appendix A of this report. Response from the U.S. Army Corps of Engineers indicates that this project crosses the Corps of Engineers’ project, streams Big and Colewa Creeks. The Corps of Engineers will need to review the plans and specifications for this structure. Response from the USGS indicates no significant improvements would be impacted by this project. To date, no response has been received from the NRCS; however, once the response is received, it will be included in the report. The letters and responses follow.

McMANUS CONSULTING ENGINEERS

KENNETH C. McMANUS, P.E.

P.O. Box 4318
MONROE, LOUISIANA 71211
PHONE: (318) 343-5600, 343-5460
FAX: (318) 343-5717



September 2, 1997

Department of the Army
Vicksburg District, Corps of Engineers
2101 North Frontage Road
Vicksburg, MS 39180-5191

Attn: Mr. Roy O. Smith, Deputy District Engineer
for Project Management

Re: SPN 713-42-01(13-21)(Const.)
SPN 700-42-0103 (Engr.)
FAP No. BRO-002S(622-630)
Off-System Bridge Replacement
Richland Parish, Louisiana

Dear Mr. Smith:

The Louisiana Department of Transportation and Development intends to replace the following bridges as a part of the State of Louisiana Off-System Bridge Replacement Program. Inquiries are being mailed to all agencies who may have planned or pending improvements that could be affected by the proposed work.

<u>Project No.</u>	<u>Name of Bridge</u>	<u>Structure No.</u>
1. S.P. 713-42-0113	Little Creek Bridge	P42-32196-91474-1
2. S.P. 713-42-0114	Jones Cutoff Road Bridge	P42-32146-91470-1
3. S.P. 713-42-0115	Brown Road Bridge	P42-32201-91506-1
4. S.P. 713-42-0116	Bee Bayou Road Bridge	P42-32240-91418-1
5. S.P. 713-42-0117	Little Road Bridge	P42-32252-91385-1
6. S.P. 713-42-0118	Snider Road Bridge	P42-32264-91310-1
7. S.P. 713-42-0119	Brimberry Road Bridge	P42-32285-91530-1
8. S.P. 713-42-0120	Smalling Road Bridge	P42-32313-91413-1
9. S.P. 713-42-0121	Futch Road Bridge	P42-32331-91401-1

MUNICIPAL • WATER SYSTEMS • SEWER • STREETS • ROADS • BRIDGES • PLANNING

Corps of Engineers
Vicksburg District
Page 2
September 2, 1997

As a necessary part of the design, we are requesting all, if any, recorded hydrologic data published by your agency with regard to the channels at the location of the above referenced bridges.

Please review the enclosed map which shows the locations of bridge crossings and respond by letter as to whether or not your agency has a potential project that would be impacted by the proposed construction activities.

We appreciate your assistance in this matter. Should you require any additional information, please do not hesitate to contact our office.

I remain sincerely,

McMANUS CONSULTING ENGINEERS


SALAM A. KNAN, E.I.T.

SAK:jb
xc: Richland Parish Police Jury
File

Mailed 9/3



REPLY TO
ATTENTION OF:

DEPARTMENT OF THE ARMY

VICKSBURG DISTRICT, CORPS OF ENGINEERS
4155 CLAY STREET
VICKSBURG, MISSISSIPPI 39180-3435

September 25, 1997

*Richland
Bridges*

Programs and Project Management Division

Mr. Kenneth C. McManus, P.E.
McManus Consulting Engineers
P.O. Box 4318
Monroe, Louisiana 71211

Dear Mr. McManus:

The following information is provided in response to your letter of September 2, 1997, concerning State Project No. 713-42-01 (13-21), Off System Bridge Replacement Program, Richland Parish, Louisiana. *9-28-97 K*

We have reviewed our project files, and it appears that one structure, No. P42-32252-91385-1, Little Road Bridge crosses a project stream, Big and Colewa Creeks. Therefore, we will need to review the Plans and Specifications for this structure. The proposed construction of the other eight bridges do not appear to adversely impact any of our current or planned projects.

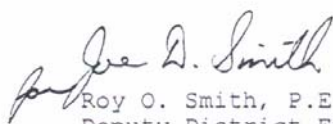
Based on the information furnished, it appears that Department of the Army permit requirements for the proposed bridge construction projects may be authorized by Nationwide Permit No. 23 for categorically excluded Department of Transportation actions as specified in the December 13, 1996, Federal Register, Final Notice of Issuance, Reissuance, and Modification of Nationwide Permits (61 FR 65874), provided the bridge projects are determined to be categorical exclusions by the Federal Highway Administration. Proof that these projects have been determined to be categorical exclusions must be provided to our office prior to authorization under this Nationwide Permit. The documentation should be sent to the following address: U.S. Army Corps of Engineers, Vicksburg District, 4155 Clay Street, Vicksburg, Mississippi, 39180, Attention: Mr. Michael F. McNair, R.F., Chief, Permit Section, Regulatory Branch.

Printed on  Recycled Paper

-2-

Thank you for advising us of these projects. Please contact Mr. McNair if you have any questions specific to our permit related comments. You may also wish to contact Mrs. Susan Smith, Louisiana, Ouachita River, and Red River Backwater Project Manager, if you have any questions of a general nature. The telephone numbers of Mr. McNair and Mrs. Smith are (601) 631-5721 and (601) 631-5494, respectively.

Sincerely,


Roy O. Smith, P.E.
Deputy District Engineer
for Project Management



United States Department of the Interior

GEOLOGICAL SURVEY
514 East Georgia Street
Ruston, Louisiana 71270
(318) 251-9630



September 24,, 1997

Salam A. Khan, EIT
McManus Consulting Engineers
P.O. Box 4318
Monroe, Louisiana 71211

RE: Louisiana Department of Transportation and Development
Off-System Bridge Replacement Program
Richland Parish, Louisiana
SPN: 713-42-01 (Const.)
SPN: 700-42-0103 (Engr.)
FAP: BRO-002S (622-630)

9-26-97
K

Dear Mr. Khan:

I have reviewed the documents sent from your office. At this time, the U.S. Geological Survey, Louisiana District does not have any proposed or existing projects that would be affected by these bridge replacements. Furthermore, we do not have any historical records pertaining to these sites. Thank you for your inquiry.

Sincerely,

A handwritten signature in black ink, appearing to read "B. D. McGee".

Benton D. McGee,
Supervisory Hydrologist

C. Potential for Watershed Development

Based on the present rural nature of the contributing watershed and its considerable distance from any existing large metropolitan area, no significant urban development is expected to take place within the watershed boundaries during the next 20 years.

D. Utility Conflicts

No major utility conflicts are anticipated when constructing the new bridge.

E. Detour Distance

The Little Creek Bridge is located on a parish gravel road with low ADT which precludes the creation of any serious detour problems for the area residents should this road be temporarily closed. The shortest detour route by interconnecting gravel roads is approximately 4.97 miles. Therefore, a detour structure may not be required during the construction of this project.

F. Average Daily Traffic

According to Traffic Data collected as of January 07, 2005, the estimated current (2005) and future (2025) Average Daily Traffic volume is 100 and 150 vehicles per day respectively. The Functional Class of this road is Rural Minor Collector, and the traffic volume makes this road an RL-1 Type Rural Road. The Traffic Data is included in Appendix B.

G. Structure Alignment

As shown in the plan view of the bridge site, the existing structure crosses the channel at approximately a 60° angle. The upstream and downstream channel is not well defined. It is assumed that the water will get out of the channel banks at a low discharge; therefore, a skewed bridge is not necessary.

H. Existing Erosion Problems/Scour Potential

There was no evidence of any existing or potential scouring from the site visit. The depth of scour for the culvert alternates calculated in the HYDR1120 run was less than what is required for headwalls.

I. Design Criteria

The lowest point of the roadway is at an elevation of 64.48 ft and is overtopped at a frequency less than two years. Therefore, all proposed alternates will be designed to pass a 5-year frequency discharge.

The backwater range of 0.1' to 0.5' for when the overtopping (or the 5-year) discharge governs the design will be used. However, investigating the upstream watershed, which is mainly covered with farmland and woods, it is assumed that the backwater would not be a significant problem should a higher backwater value be considered for the overtopping year discharge.

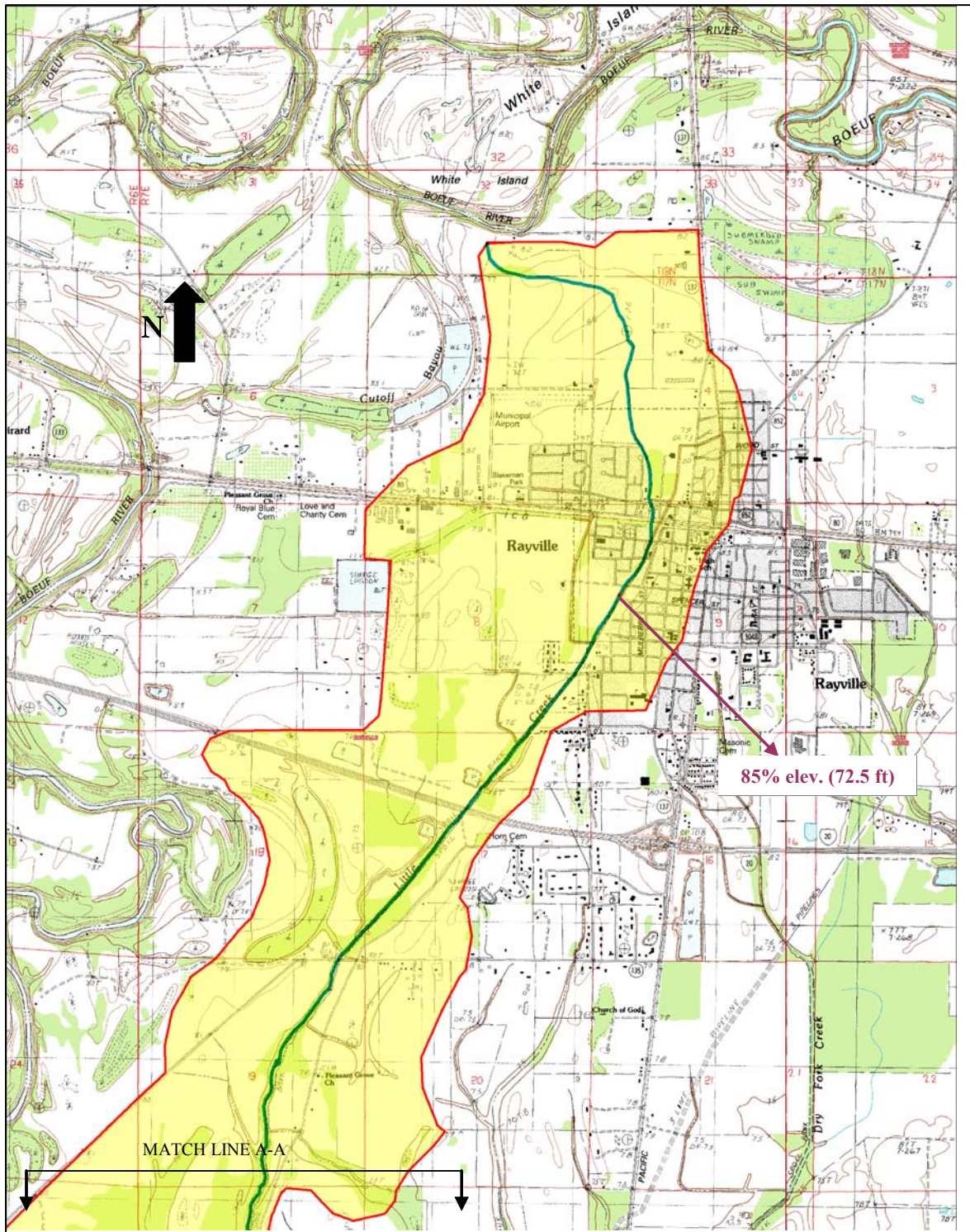
IV. DESIGN ANALYSIS

A. Hydrology

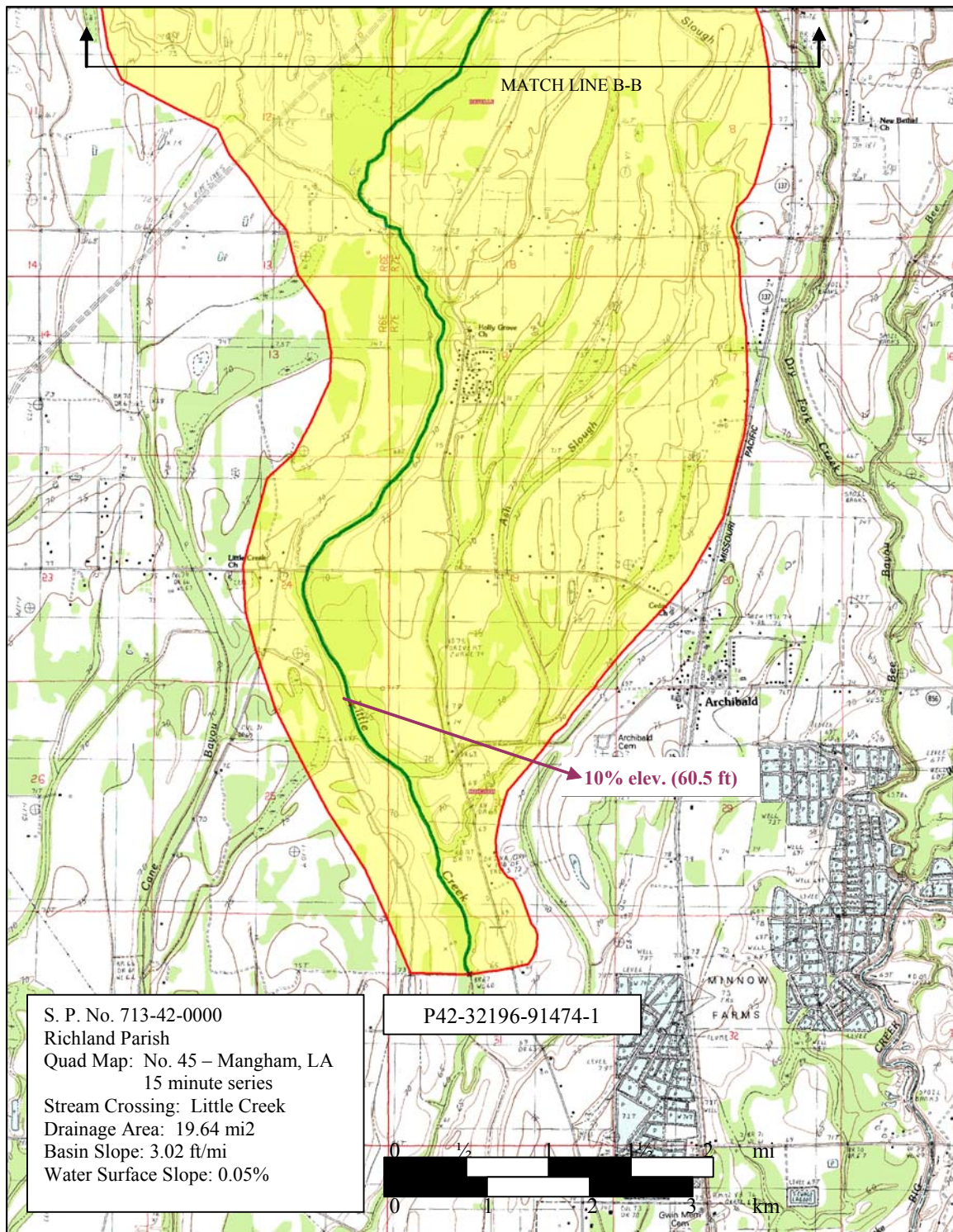
The contributing watershed of the bridge site consists of almost flat terrain with agricultural lands, forests, pastures and scattered home sites. The total drainage area is 19.64 mi². The basin slope was calculated to be 3.02 ft/mi. The hydraulic length is 27400 ft; the 10% elevation is equal to 72.5 ft, and the 85% elevation is equal to 60.5 ft. The effect of urbanization on the watershed is considered to be negligible.

The USGS method in HYDR1130 was used to determine the peak discharges for the bridge site, and the data then plotted on probability paper for discharge versus frequency. The final results are shown on the following table. The drainage area map, output from HDYR1130 and the discharge - frequency curve are included in this section.

FREQUENCY	DISCHARGE
2-year	660 cfs
5-year	1000 cfs
10-year	1250 cfs
25-year	1600 cfs
50-year	1850 cfs
100-year	2100 cfs
500-year	2800 cfs







LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
HYDRAULICS SECTION

HYDR1130-071498

DESIGNER: Designer Name

DATE: 04-08-2005

REMARKS: RICHLAND PARISH - LITTLE CREEK BRIDGE ON TARVER ROAD

STATE PROJECT NUMBER 713-42-0000

USGS PEAK DISCHARGE

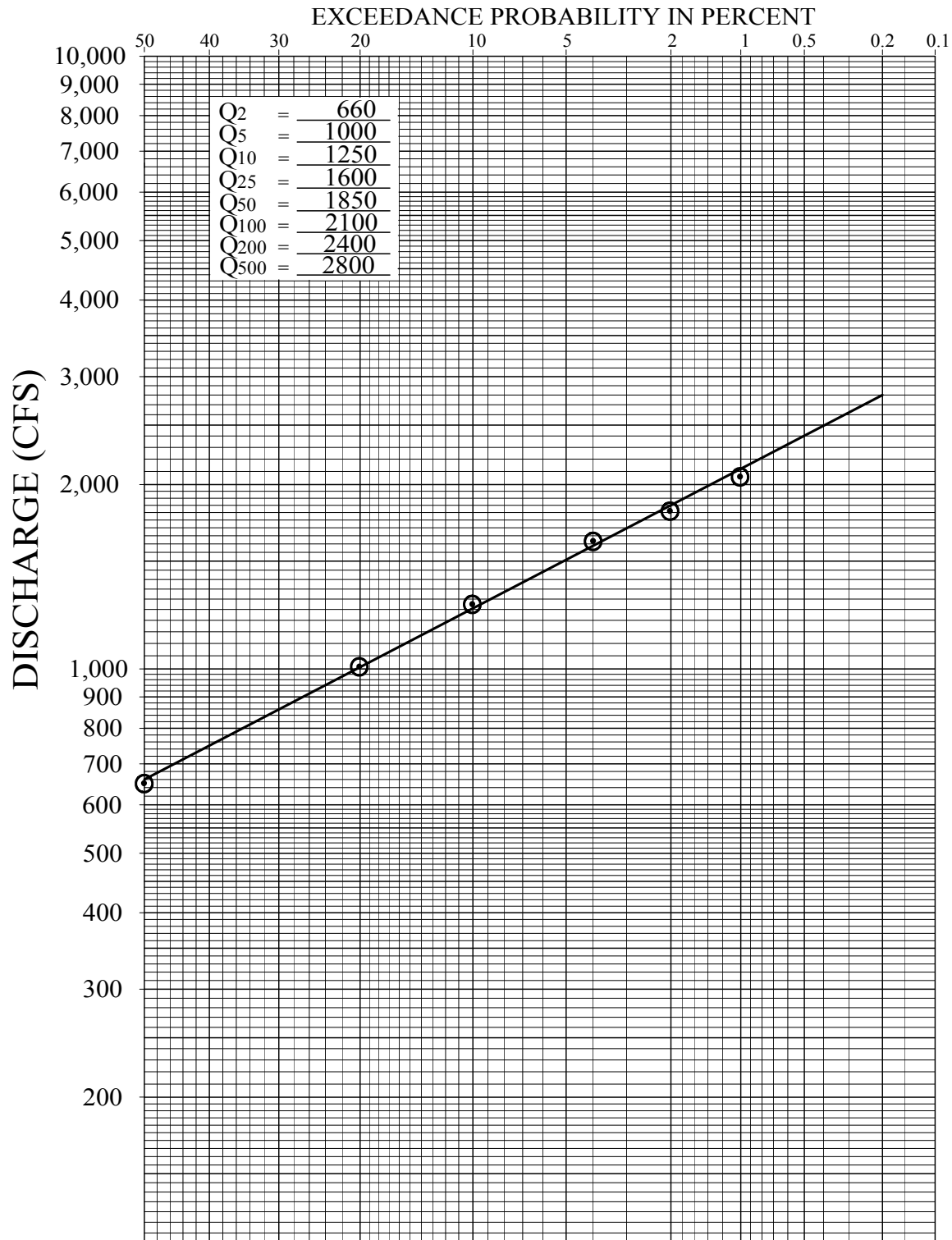
STATION	Lit_Creek
DRAINAGE AREA (SQ. MI.)	19.64
URBAN ADJUSTMENT RATIO	1.00
SLOPE (FT./MI.)	3.02
MEAN ANNUAL PRECIPITATION (IN.)	52.00

Q2 (CFS)	651.
Q5 (CFS)	1008.
Q10 (CFS)	1286.
Q25 (CFS)	1616.
Q50 (CFS)	1820.
Q100 (CFS)	2067.

DISCHARGE - FREQUENCY CURVE

STATE PROJECT 713-42-0000
ROUTE Tarver Road
PARISH Richland

STREAM NAME Little Creek
STRUCTURE NO 42-32196-91474-1
BY: Name DATE: July 2004



B. Hydraulics

The FHWA's Computer Program "WSPRO - A Computer Model for Water Surface Profile Computations" (HYDRAIN 6.0 version) was used to determine the water surface elevations for the 2, 5, 10, 25, 50, and 100 year discharges. (The September 1990 publication of the *User's Manual for WSPRO* was also consulted for background information of the program.) A representative floodplain cross-section was constructed utilizing field survey information and the USGS topographic maps. The floodplain roughness coefficients were estimated from the USGS quadrangle maps, the *LA DOTD Hydraulics Manual*, and the FHWA publication, *Guide for Selecting Manning's Roughness Coefficients for Natural Channels and Flood Plains*. The water surface slope was determined from the USGS quadrangle map. The average water surface slope of the channel was determined to be 0.05 % (percent). To calculate the water surface profile, fixed geometry mode was considered for the existing bridge and design mode was considered for the proposed bridge.

A stage-discharge curve was plotted using the stage values from the WSPRO output for the existing bridge. WSPRO was run without the roadway card because the overtopping was in such excess the program did not calculate the amount of discharge going through the bridge for all frequencies. The water surface elevation and the backwater represent all the flow going through the bridge opening. A linear relationship was assumed to find the overtopping frequency. The lowest point of the roadway, 64.48 ft was lower than the 2-year stage; therefore, the 5-year discharge was used in the design for the proposed structure alternates.

For the bridge options, the area of opening for the design year discharge was hand-calculated considering the required excavation at the bridge. The water surface elevation in this hand-calculation was assumed as the unconstricted water surface at the approach section or the low chord elevation, whichever was lower. The hand-calculated area was compared to the area given by WSPRO to make sure they were within 10% of each other.

The hand-calculated area value was put on the Hydraulic Data Table. The velocity on the Hydraulic Data Table was determined by dividing the design discharge by the hand-calculated area of opening. The calculated velocity is based on the assumption that all the design discharge would pass through the bridge without considering any road overflow. However, in the actual field situation, this may not be true because of overtopping of a portion of discharge over the road. For the proposed bridge alternate, there is pressure flow through the bridge. The area of opening calculation follows the WSPRO output of the proposed bridge alternate.

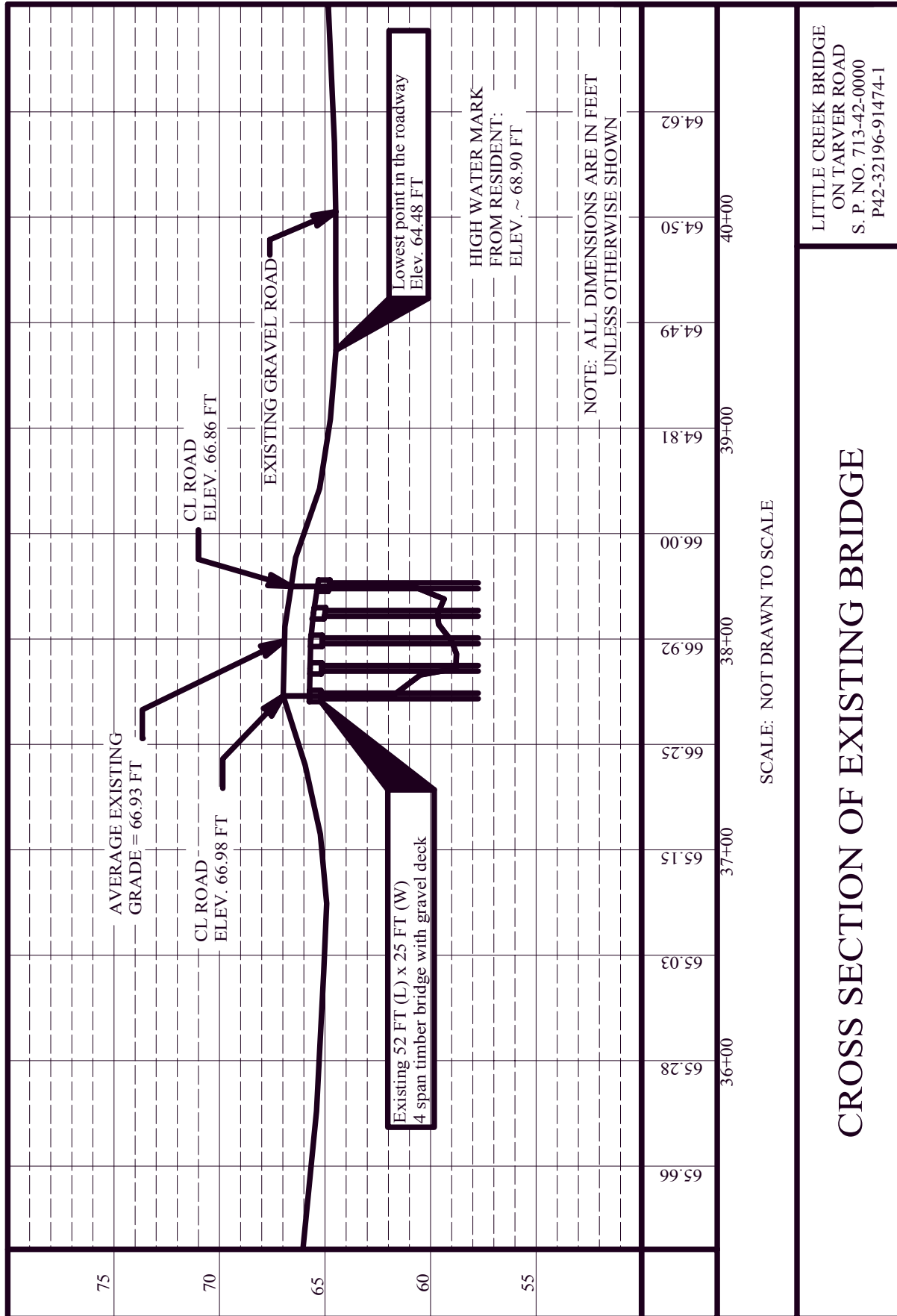
A 3-span (60 ft long) bridge and a 4-span (80 ft long) bridge were both considered. Because of the amount of fill that would be required for the 3-span bridge, the 4-span bridge was considered as the preferable alternate. Only the analysis for the 4-span bridge is included in the report.

B. Hydraulics Contd.

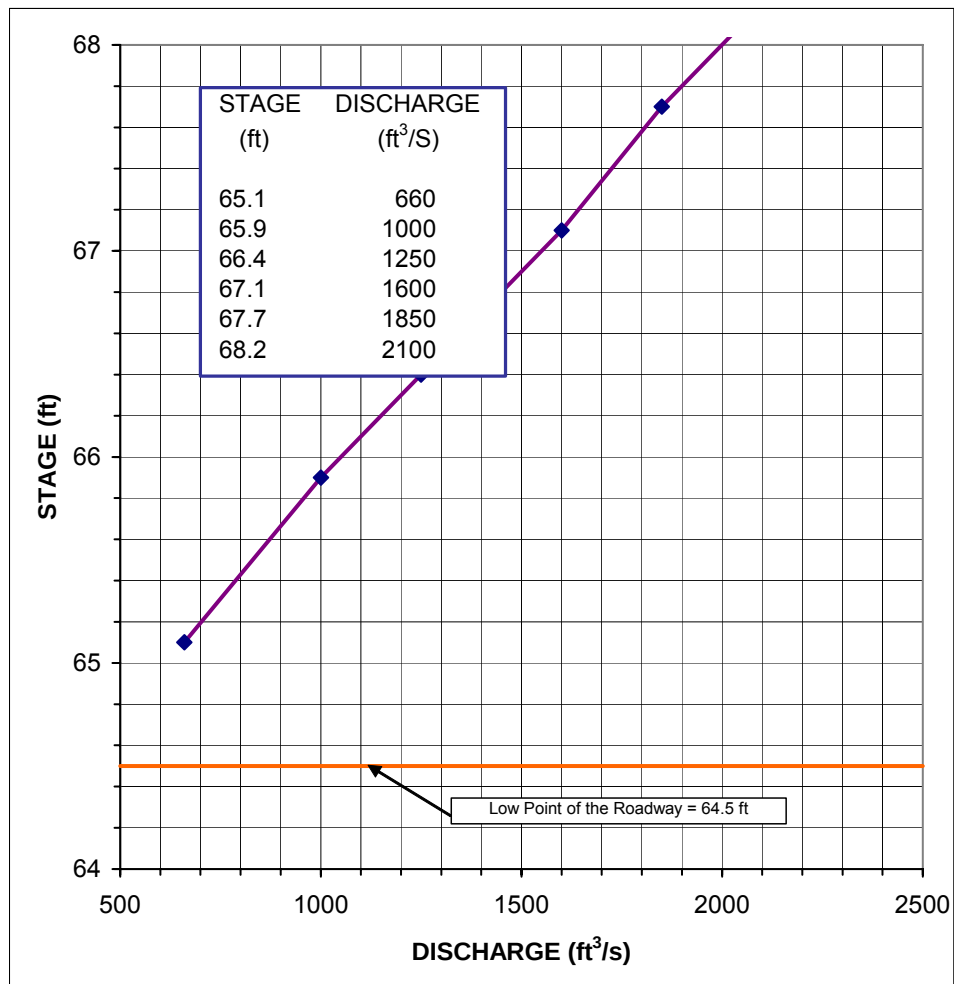
To investigate culvert alternates, LA DOTD's HYDR1120 (July 1997 version) was used to calculate the backwater. To meet the construction clearance requirement, the maximum size round pipe had to be less than 6.5' or 7.5' if the pipe were buried. Because of headroom clearance limitations, arch pipe was used for the analysis. The tailwater depth was determined using the output from the WSPRO analysis for the existing bridge. HYDR1120 computations follow the WSPRO calculations.

The slope used in the HYDR1120 program was determined from the survey of the natural channel slope for the length of the pipe.

For the Precast 3 Sided Structure, a size was determined from a manufacturer's listing and WSPRO was used to analyze the structure. A 2 – 28' × 7' was chosen. However, the size used in WSPRO was based on an equivalent area of opening of 167 ft² per barrel taken from the manufacturer's size list. In WSPRO, the size is 2 – 27.6' × 6.05' which gives an area of 334 ft². It was run as a culvert, since it will presumably have a concrete slab bottom as per LA DOTD requirements. However, if it is decided to use this alternate, any changes made to the P3S structure can be reanalyzed if necessary.



Little Creek at Tarver Road
S.P. No. 713-42-0000 Richland Parish



Stage - Discharge Curve

**WSPRO Calculations
for the
Existing 52.0 ft Timber Bridge**

```

*
T1      S.P. No. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE
*
*          Q2      Q5      Q10      Q25      Q50      Q100
Q        660      1000     1250     1600     1850     2100
SK       .0005     .0005     .0005     .0005     .0005     .0005
*
XT      TEMP 5000
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GR          3606,63.09  3655,64.14  3693,64.17
GR          3734,62.04  3767,61.41  3799,59.19
GR          3831,60.92  3836,62.20  3852,64.14
GR          3856,64.27  3954,62.34  4053,63.71
GR          4149,64.04  4149,69.00
*
XS      EXIT 4948  *  *  *  .0005
GT
N          0.100      0.085      0.100
SA          3767      3831
*
XS      FULL 5000  *  *  *  .0005
*
BR      BRDG 5000  65.69  *  *  .0005
GR          3773,65.69  3773,61.80  3783,60.48
GR          3786,58.80  3793,58.40  3799,59.00
GR          3807,59.62  3812,59.66  3819,59.36
GR          3824,60.57  3825,60.95  3825,65.69
GR          3773,65.69
N          0.085
CD          2  20.00  2  66.93
AB          61.80  60.95
*
XS      APPR 5072  *  *  *  .0005
*
EX
ER

```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time:  4/13/2005  10:23 am    Version V200104
Input File: SR-EBR-E      Output File: SR-EBR-E.LST
*-----*
T1      S.P. NO. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE
Q        660    1000    1250    1600    1850    2100

***      Processing Flow Data; Placing Information into Sequence  1      ***

SK        .0005  .0005  .0005  .0005  .0005  .0005
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***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

* Starting To Process Header Record TEMP *

XT	TEMP 5000			
GR	3449,69.00	3449,66.90	3508,65.29	
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GR	3734,62.04	3767,61.41	3799,59.19	
GR	3831,60.92	3836,62.20	3852,64.14	
GR	3856,64.27	3954,62.34	4053,63.71	
GR	4149,64.04	4149,69.00		

*** Completed Reading Data Associated With Header Record TEMP ***
*** Storing Template Header Record Data In Memory ***

*** Data Summary For Header Record TEMP ***
SRD Location: 5000. Valley Slope: ***** Error Code 0

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.000	3449.100	66.900	3508.000	65.290
3606.000	63.090	3655.000	64.140	3693.000	64.170
3734.000	62.040	3767.000	61.410	3799.000	59.190
3831.000	60.920	3836.000	62.200	3852.000	64.140
3856.000	64.270	3954.000	62.340	4053.000	63.710
4149.000	64.040	4149.100	69.000		

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #17 increased to eliminate vertical segment.

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.000)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.000)
Minimum Y-Elevation: 59.190 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.000 (associated X-Station: 3449.000)

* Finished Processing Header Record TEMP *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

* Starting To Process Header Record EXIT *

XS EXIT 4948 * * * .0005
GT
N 0.100 0.085 0.100
SA 3767 3831

*** Completed Reading Data Associated With Header Record EXIT ***
*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***
SRD Location: 4948. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	68.974	3449.100	66.874	3508.000	65.264
3606.000	63.064	3655.000	64.114	3693.000	64.144
3734.000	62.014	3767.000	61.384	3799.000	59.164
3831.000	60.894	3836.000	62.174	3852.000	64.114
3856.000	64.244	3954.000	62.314	4053.000	63.684
4149.000	64.014	4149.100	68.974		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 68.974)
Maximum X-Station: 4149.100 (associated Y-Elevation: 68.974)
Minimum Y-Elevation: 59.164 (associated X-Station: 3799.000)
Maximum Y-Elevation: 68.974 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
		3767.000
2	.085	---
		3831.000
3	.100	---

* Finished Processing Header Record EXIT *

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***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

*-----*
* Starting To Process Header Record FULL *
*-----*

XS FULL 5000 * * * .0005

*** Completed Reading Data Associated With Header Record FULL ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULL ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)
X Y X Y X Y
-----
3449.000 69.000 3449.100 66.900 3508.000 65.290
3606.000 63.090 3655.000 64.140 3693.000 64.170
3734.000 62.040 3767.000 61.410 3799.000 59.190
3831.000 60.920 3836.000 62.200 3852.000 64.140
3856.000 64.270 3954.000 62.340 4053.000 63.710
4149.000 64.040 4149.100 69.000
-----

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 ( associated Y-Elevation: 69.000 )
Maximum X-Station: 4149.100 ( associated Y-Elevation: 69.000 )
Minimum Y-Elevation: 59.190 ( associated X-Station: 3799.000 )
Maximum Y-Elevation: 69.000 ( associated X-Station: 3449.000 )

Roughness Data ( 3 SubAreas )
SubArea Roughness Coefficient Horizontal Breakpoint
-----
1 .100 ---
--- 3767.000
2 .085 ---
--- 3831.000
3 .100 ---
-----

*-----*
* Finished Processing Header Record FULL *
*-----*

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***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

* Starting To Process Header Record BRDG *

BR BRDG 5000 65.69 * * .0005
GR 3773,65.69 3773,61.80 3783,60.48
GR 3786,58.80 3793,58.40 3799,59.00
GR 3807,59.62 3812,59.66 3819,59.36
GR 3824,60.57 3825,60.95 3825,65.69
GR 3773,65.69
N 0.085
CD 2 20.00 2 66.93
AB 61.80 60.95

*** Completed Reading Data Associated With Header Record BRDG ***
*** Storing Bridge Data In Temporary File As Record Number 3 ***

*** Data Summary For Bridge Record BRDG ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (13 pairs)					
X	Y	X	Y	X	Y
3773.000	65.690	3773.100	61.800	3783.000	60.480
3786.000	58.800	3793.000	58.400	3799.000	59.000
3807.000	59.620	3812.000	59.660	3819.000	59.360
3824.000	60.570	3825.000	60.950	3825.100	65.690
3773.000	65.690				

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #12 increased to eliminate vertical segment.

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3773.000 (associated Y-Elevation: 65.690)
Maximum X-Station: 3825.100 (associated Y-Elevation: 65.690)
Minimum Y-Elevation: 58.400 (associated X-Station: 3793.000)
Maximum Y-Elevation: 65.690 (associated X-Station: 3773.000)

Roughness Data (1 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.085	---

Discharge coefficient parameters
BRTYPE BRWidth EMBSS EMBELv UserCD
2 20.000 2.00 66.930 *****

Pressure flow elevations
AVBCEL PFElev
***** 65.690

Abutment Parameters
ABSLPL ABSLPR XTOELT YTOELT XTOERT YTOERT
61.800 60.950 ***** ***** ***** *****

** No Pier/Pile Data Encountered **

* Finished Processing Header Record BRDG *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

* Starting To Process Header Record APPR *

XS APPR 5072 * * * .0005

*** Completed Reading Data Associated With Header Record APPR ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***
SRD Location: 5072. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.036	3449.100	66.936	3508.000	65.326
3606.000	63.126	3655.000	64.176	3693.000	64.206
3734.000	62.076	3767.000	61.446	3799.000	59.226
3831.000	60.956	3836.000	62.236	3852.000	64.176
3856.000	64.306	3954.000	62.376	4053.000	63.746
4149.000	64.076	4149.100	69.036		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.036)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.036)
Minimum Y-Elevation: 59.226 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.036 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	3767.000
2	.085	---
	---	3831.000
3	.100	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

* Finished Processing Header Record APPR *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

EX

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* Summary of Boundary Condition Information *

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#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	660.00	*****	.0005	Sub-Critical
2	1000.00	*****	.0005	Sub-Critical
3	1250.00	*****	.0005	Sub-Critical
4	1600.00	*****	.0005	Sub-Critical
5	1850.00	*****	.0005	Sub-Critical
6	2100.00	*****	.0005	Sub-Critical

=====

* Beginning 6 Profile Calculation(s) *

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***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 1 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	64.811	.011	660.000	1093.243	*****	3528.173
Header Type: XS	64.822	*****	.604	29493.17	*****	4149.016
SRD: 4948.000	61.678	*****	.109	*****	1.853	*****
Section: FULL	64.838	.010	660.000	1093.779	52.000	3528.134
Header Type: FV	64.848	.026	.603	29512.04	52.000	4149.016
SRD: 5000.000	61.704	.000	.109	.0005	1.853	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	64.875	.010	660.000	1094.516	72.000	3528.081
Header Type: AS	64.886	.036	.603	29538.05	72.000	4149.016
SRD: 5072.000	61.740	.000	.109	.0005	1.852	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	64.825	.160	660.000	268.259	52.000	3773.022
Header Type: BR	64.985	.060	2.460	12782.79	52.000	3825.082
SRD: 5000.000	61.323	.103	.249	*****	1.697	-.002

Bridge Summary Information - Coordinate Mode

Flow Class: 1 - Free-surface flow with no embankment overtopping
Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLEN	XLAB	XRAB
.7677	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

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-
Unconstricted Full Valley Section Water Surface Elevation:      64.838
Downstream Bridge Section Water Surface Elevation:             64.825
Bridge DrawDown Distance:                                     .013
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-

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	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	65.121	.008	660.000	1248.746	52.000	3517.114
Header Type: AS	65.129	.070	.529	35190.84	72.853	4149.021
SRD: 5072.000	61.740	.074	.087	.0005	1.739	-.010

** Change in Approach Section Water Surface Elevation: .246 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.916	.612	13761.0	3778.889	3830.948	65.121

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 1 >>

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 2 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	65.430	.011	1000.000	1485.961	*****	3501.922
Header Type: XS	65.441	*****	.673	44691.14	*****	4149.029
SRD: 4948.000	62.195	*****	.099	*****	1.603	*****
Section: FULL	65.457	.011	1000.000	1486.717	52.000	3501.880
Header Type: FV	65.469	.026	.673	44723.01	52.000	4149.029
SRD: 5000.000	62.221	.000	.099	.0005	1.603	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	65.495	.011	1000.000	1487.716	72.000	3501.823
Header Type: AS	65.506	.036	.672	44765.10	72.000	4149.029
SRD: 5072.000	62.257	.000	.099	.0005	1.602	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>
<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===220 FLOW CLASS 1 (4) SOLUTION INDICATES POSSIBLE PRESSURE FLOW.
WS3, WSIU, WS1, PFELV: 65.43 65.84 65.93 65.69

===245 ATTEMPTING FLOW CLASS 2 (5) SOLUTION.

===250 INSUFFICIENT HEAD FOR PRESSURE FLOW.
YU/Z, WSIU, WS: 1.04 65.91 66.03

===270 REJECTED FLOW CLASS 2 (5) SOLUTION.

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.429	.314	1000.000	299.716	52.000	3773.007
Header Type: BR	65.743	.077	3.336	15173.10	52.000	3825.094
SRD: 5000.000	61.930	.225	.330	*****	1.815	.000

Bridge Summary Information - Coordinate Mode

Flow Class: 1 - Free-surface flow with no embankment overtopping

Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLN	XLAB	XRAB
.7424	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

Unconstricted Full Valley Section Water Surface Elevation: 65.457
Downstream Bridge Section Water Surface Elevation: 65.429
Bridge DrawDown Distance: .028

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	65.933	.007	1000.000	1775.104	52.000	3485.778
Header Type: AS	65.941	.089	.563	57498.69	77.455	4149.038
SRD: 5072.000	62.257	.109	.074	.0005	1.485	.001

** Change in Approach Section Water Surface Elevation: .439 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
.920	.699	17295.1	3781.481	3833.569	65.933

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 2 >>

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 3 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	65.818	.012	1250.000	1739.512	*****	3487.744
Header Type: XS	65.830	*****	.719	55856.38	*****	4149.037
SRD: 4948.000	62.523	*****	.096	*****	1.497	*****
Section: FULL	65.845	.012	1250.000	1740.341	52.000	3487.698
Header Type: FV	65.857	.026	.718	55894.41	52.000	4149.037
SRD: 5000.000	62.549	.000	.096	.0005	1.497	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	65.883	.012	1250.000	1741.415	72.000	3487.638
Header Type: AS	65.895	.036	.718	55943.72	72.000	4149.037
SRD: 5072.000	62.585	.000	.095	.0005	1.496	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 65.84 65.69

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.690	.254	1266.383	313.301	52.000	3773.000
Header Type: BR	65.944	*****	4.042	10781.55	*****	3825.100
SRD: 5000.000	62.316	*****	.411	*****	1.000	*****

Bridge Summary Information - Coordinate Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLN	XLAB	XRAB
.8000	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

-
Unconstricted Full Valley Section Water Surface Elevation: 65.845
Downstream Bridge Section Water Surface Elevation: 65.690
Bridge DrawDown Distance: .155

-

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	66.393	.008	1250.000	2084.059	52.000	3468.950
Header Type: AS	66.401	.160	.600	72475.01	79.964	4149.047
SRD: 5072.000	62.585	.109	.071	.0005	1.399	.013

** Change in Approach Section Water Surface Elevation: .511 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	66.393

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 3 >>

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 4 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	66.304	.013	1600.000	2065.732	*****	3469.937
Header Type: XS	66.318	*****	.775	71551.91	*****	4149.046
SRD: 4948.000	62.963	*****	.093	*****	1.404	*****
Section: FULL	66.332	.013	1600.000	2066.537	52.000	3469.893
Header Type: FV	66.345	.026	.774	71592.38	52.000	4149.046
SRD: 5000.000	62.989	.000	.093	.0005	1.403	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	66.369	.013	1600.000	2067.555	72.000	3469.838
Header Type: AS	66.382	.036	.774	71643.56	72.000	4149.046
SRD: 5072.000	63.025	.000	.093	.0005	1.403	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 66.33 65.69

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.690	.395	1578.422	313.301	52.000	3773.000
Header Type: BR	66.085	*****	5.038	10781.55	*****	3825.100
SRD: 5000.000	62.721	*****	.512	*****	1.000	*****

Bridge Summary Information - Coordinate Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLN	XLAB	XRAB
.8000	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

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-----
-
Unconstricted Full Valley Section Water Surface Elevation:      66.332
Downstream Bridge Section Water Surface Elevation:              65.690
Bridge DrawDown Distance:                                       .642
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-

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	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	67.134	.008	1600.000	2597.398	52.000	3449.091
Header Type: AS	67.142	.195	.616	100402.60	82.342	4149.062
SRD: 5072.000	63.025	.109	.064	.0005	1.302	-.013

** Change in Approach Section Water Surface Elevation: .765 **

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      Approach Section APPR Flow Contraction Information
      M( G )   M( K )       KQ       XLKQ       XRKQ       OTEL
-----
***** ***** ***** ***** ***** *****
                                     67.134
-----

```

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 4 >>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 5 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	66.620	.014	1850.000	2282.112	*****	3458.379
Header Type: XS	66.634	*****	.811	82714.47	*****	4149.053
SRD: 4948.000	63.060	*****	.092	*****	1.359	*****
Section: FULL	66.648	.014	1850.000	2283.026	52.000	3458.330
Header Type: FV	66.662	.026	.810	82762.80	52.000	4149.053
SRD: 5000.000	63.086	.000	.092	.0005	1.359	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	66.685	.014	1850.000	2284.167	72.000	3458.270
Header Type: AS	66.699	.036	.810	82823.14	72.000	4149.053
SRD: 5072.000	63.122	.000	.092	.0005	1.359	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 66.65 65.69

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.690	.538	1843.311	313.301	52.000	3773.000
Header Type: BR	66.228	*****	5.884	10781.55	*****	3825.100
SRD: 5000.000	63.065	*****	.598	*****	1.000	*****

Bridge Summary Information - Coordinate Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLN	XLAB	XRAB
.8000	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

-
Unconstricted Full Valley Section Water Surface Elevation: 66.648
Downstream Bridge Section Water Surface Elevation: 65.690
Bridge DrawDown Distance: .958

-

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	67.693	.007	1850.000	2988.056	52.000	3449.064
Header Type: AS	67.700	.212	.619	124965.10	83.626	4149.073
SRD: 5072.000	63.122	.109	.059	.0005	1.242	-.004

** Change in Approach Section Water Surface Elevation: 1.007 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	67.693

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 5 >>

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
EXISTING 4-SPAN, 52 FEET TIMBER BRIDGE

<< Beginning Computations for Profile 6 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	66.914	.015	2100.000	2486.448	*****	3449.098
Header Type: XS	66.929	*****	.845	93846.28	*****	4149.059
SRD: 4948.000	63.444	*****	.091	*****	1.324	*****
Section: FULL	66.941	.015	2100.000	2487.323	52.000	3449.098
Header Type: FV	66.956	.026	.844	93897.27	52.000	4149.059
SRD: 5000.000	63.470	.000	.091	.0005	1.324	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	66.979	.015	2100.000	2488.382	72.000	3449.098
Header Type: AS	66.993	.036	.844	93958.96	72.000	4149.059
SRD: 5072.000	63.506	.000	.091	.0005	1.324	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 66.94 65.69

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.690	.697	2098.123	313.301	52.000	3773.000
Header Type: BR	66.387	*****	6.697	10781.55	*****	3825.100
SRD: 5000.000	63.359	*****	.681	*****	1.000	*****

Bridge Summary Information - Coordinate Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 2 - Sloping embankments & vertical abutments w/o wingwalls

C	PFELEV	BLN	XLAB	XRAB
.8000	65.690	*****	*****	*****

No Pier(s)/Pile(s) Present at Bridge

```
-----
-
Unconstricted Full Valley Section Water Surface Elevation: 66.941
Downstream Bridge Section Water Surface Elevation: 65.690
Bridge DrawDown Distance: 1.251
-----
-
```

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	68.250	.007	2100.000	3378.585	52.000	3449.037
Header Type: AS	68.258	.228	.622	151730.70	84.700	4149.084
SRD: 5072.000	63.506	.109	.055	.0005	1.199	-.001

** Change in Approach Section Water Surface Elevation: 1.272 **

```

Approach Section APPR Flow Contraction Information
M( G ) M( K ) KQ XLKQ XRKQ OTEL
-----
***** ***** ***** ***** ***** *****
68.250
-----

```

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 6 >>

ER

```

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 1 Seconds *****

```

**WSPRO Calculations
for the
Proposed 4 span (80 ft)
Slab Span Bridge**

```

*
T1      S.P. No. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE
*
*      Q5      Q100
Q      1000    2100
SK     .0005   .0005
*
XT      TEMP 5000
GR      3449,69.00  3449,66.90  3508,65.29
GR      3606,63.09  3655,64.14  3693,64.17
GR      3734,62.04  3767,61.41  3799,59.19
GR      3831,60.92  3836,62.20  3852,64.14
GR      3856,64.27  3954,62.34  4053,63.71
GR      4149,64.04  4149,69.00
*
XS      EXIT 4920  *  *  *  .0005
GT
N      0.100      0.085      0.100
SA      3767      3831
*
XS      FULL 5000  *  *  *  .0005
*
BR      BRDG 5000  65.26  *  *  .0005
GR      3761.17,63.93  3777.76,58.40
GR      3822.24,58.40  3838.83,63.93
BC      65.26
BL      80      3800      3800
N      0.085
CD      3  20.00  3  66.93
AB      3.0
*
XS      APPR 5100  *  *  *  .0005
*
EX
ER

```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 5/12/2005 2:25 pm Version V200104
Input File: SR-PBR-E Output File: SR-PBR-E.LST
*-----*
T1      S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE
Q        1000    2100

*** Processing Flow Data; Placing Information into Sequence 1 ***

SK      .0005 .0005
```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

* Starting To Process Header Record TEMP *

XT	TEMP 5000			
GR	3449,69.00	3449,66.90	3508,65.29	
GR	3606,63.09	3655,64.14	3693,64.17	
GR	3734,62.04	3767,61.41	3799,59.19	
GR	3831,60.92	3836,62.20	3852,64.14	
GR	3856,64.27	3954,62.34	4053,63.71	
GR	4149,64.04	4149,69.00		

*** Completed Reading Data Associated With Header Record TEMP ***
*** Storing Template Header Record Data In Memory ***

*** Data Summary For Header Record TEMP ***
SRD Location: 5000. Valley Slope: ***** Error Code 0

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.000	3449.100	66.900	3508.000	65.290
3606.000	63.090	3655.000	64.140	3693.000	64.170
3734.000	62.040	3767.000	61.410	3799.000	59.190
3831.000	60.920	3836.000	62.200	3852.000	64.140
3856.000	64.270	3954.000	62.340	4053.000	63.710
4149.000	64.040	4149.100	69.000		

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #17 increased to eliminate vertical segment.

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.000)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.000)
Minimum Y-Elevation: 59.190 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.000 (associated X-Station: 3449.000)

* Finished Processing Header Record TEMP *

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

*-----*
* Starting To Process Header Record EXIT *
*-----*

XS  EXIT 4920 * * * .0005
GT
N      0.100      0.085      0.100
SA      3767      3831

*** Completed Reading Data Associated With Header Record EXIT ***
*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***
SRD Location: 4920. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)
X      Y      X      Y      X      Y
-----
3449.000  68.960  3449.100  66.860  3508.000  65.250
3606.000  63.050  3655.000  64.100  3693.000  64.130
3734.000  62.000  3767.000  61.370  3799.000  59.150
3831.000  60.880  3836.000  62.160  3852.000  64.100
3856.000  64.230  3954.000  62.300  4053.000  63.670
4149.000  64.000  4149.100  68.960
-----

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 ( associated Y-Elevation: 68.960 )
Maximum X-Station: 4149.100 ( associated Y-Elevation: 68.960 )
Minimum Y-Elevation: 59.150 ( associated X-Station: 3799.000 )
Maximum Y-Elevation: 68.960 ( associated X-Station: 3449.000 )

Roughness Data ( 3 SubAreas )
SubArea  Roughness  Horizontal
          Coefficient Breakpoint
-----
1          .100      ---
          ---      3767.000
2          .085      ---
          ---      3831.000
3          .100      ---
          ---
-----

*-----*
* Finished Processing Header Record EXIT *
*-----*

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

* Starting To Process Header Record FULL *

XS FULL 5000 * * * .0005

*** Completed Reading Data Associated With Header Record FULL ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULL ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.000	3449.100	66.900	3508.000	65.290
3606.000	63.090	3655.000	64.140	3693.000	64.170
3734.000	62.040	3767.000	61.410	3799.000	59.190
3831.000	60.920	3836.000	62.200	3852.000	64.140
3856.000	64.270	3954.000	62.340	4053.000	63.710
4149.000	64.040	4149.100	69.000		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.000)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.000)
Minimum Y-Elevation: 59.190 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.000 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	3767.000
2	.085	---
	---	3831.000
3	.100	---

* Finished Processing Header Record FULL *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

* Starting To Process Header Record BRDG *

BR BRDG 5000 65.26 * * .0005
GR 3761.17, 63.93 3777.76, 58.40
GR 3822.24, 58.40 3838.83, 63.93
BC 65.26
BL 80 3800 3800
N 0.085
CD 3 20.00 3 66.93
AB 3.0

*** Completed Reading Data Associated With Header Record BRDG ***
*** Storing Bridge Data In Temporary File As Record Number 3 ***

*** Data Summary For Bridge Record BRDG ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (5 pairs)					
X	Y	X	Y	X	Y
3760.000	65.260	3780.795	58.400	3819.205	58.400
3840.000	65.260	3760.000	65.260		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3760.000 (associated Y-Elevation: 65.260)
Maximum X-Station: 3840.000 (associated Y-Elevation: 65.260)
Minimum Y-Elevation: 58.400 (associated X-Station: 3819.205)
Maximum Y-Elevation: 65.260 (associated X-Station: 3760.000)

Roughness Data (1 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.085	---

Discharge coefficient parameters				
BRTYPE	BRWidth	EMBSS	EMBElv	UserCD
3	20.000	3.00	66.930	*****

Pressure flow elevations	
AVBCel	PFElev
65.260	65.260

Abutment Parameters						
ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT	
3.000	3.000	3780.795	58.400	3819.205	58.400	

Bridge Length and Bottom Chord component input data						
BRLEN	LOCOPT	XCONLT	XCONRT	BCELEV	BCSLP	BCXSTA
80.000	0	3800.000	3800.000	65.260	.0000	3800.000

** No Pier/Pile Data Encountered **

```
*-----*
*      Finished Processing Header Record BRDG      *
*-----*
```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

* Starting To Process Header Record APPR *

XS APPR 5100 * * * .0005

*** Completed Reading Data Associated With Header Record APPR ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***
SRD Location: 5100. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.050	3449.100	66.950	3508.000	65.340
3606.000	63.140	3655.000	64.190	3693.000	64.220
3734.000	62.090	3767.000	61.460	3799.000	59.240
3831.000	60.970	3836.000	62.250	3852.000	64.190
3856.000	64.320	3954.000	62.390	4053.000	63.760
4149.000	64.090	4149.100	69.050		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.050)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.050)
Minimum Y-Elevation: 59.240 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.050 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	3767.000
2	.085	---
	---	3831.000
3	.100	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

* Finished Processing Header Record APPR *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

EX

=====

* Summary of Boundary Condition Information *

=====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	1000.00	*****	.0005	Sub-Critical
2	2100.00	*****	.0005	Sub-Critical

=====

* Beginning 2 Profile Calculation(s) *

=====

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

<< Beginning Computations for Profile 1 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	65.416	.011	1000.000	1485.965	*****	3501.922
Header Type: XS	65.427	*****	.673	44691.34	*****	4149.029
SRD: 4920.000	62.181	*****	.099	*****	1.603	*****
Section: FULL	65.458	.011	1000.000	1487.107	80.000	3501.858
Header Type: FV	65.469	.040	.672	44739.42	80.000	4149.029
SRD: 5000.000	62.221	.000	.099	.0005	1.602	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	65.510	.011	1000.000	1488.451	100.000	3501.782
Header Type: AS	65.521	.050	.672	44796.08	100.000	4149.029
SRD: 5100.000	62.271	.000	.099	.0005	1.602	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 65.46 65.26

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.260	.096	1008.243	406.146	80.000	3760.000
Header Type: BR	65.356	*****	2.482	13092.69	*****	3840.000
SRD: 5000.000	60.984	*****	.275	*****	1.000	*****

Bridge Summary Information - Component Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 3 - Sloping embankments & sloping spillthrough abutments

C	PFELEV	BLN	XLAB	XRAB
.8000	65.260	80.000	3780.795	3819.205

No Pier(s)/Pile(s) Present at Bridge

-
Unconstricted Full Valley Section Water Surface Elevation: 65.458
Downstream Bridge Section Water Surface Elevation: 65.260
Bridge DrawDown Distance: .198

-

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	65.753	.009	1000.000	1646.539	80.000	3492.907
Header Type: AS	65.761	.154	.607	51651.63	104.099	4149.034
SRD: 5100.000	62.271	.000	.084	.0005	1.532	.008

** Change in Approach Section Water Surface Elevation: .243 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	65.753

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 1 >>

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED 4-SPAN, 80 FEET SLAB SPAN BRIDGE

<< Beginning Computations for Profile 2 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	66.900	.015	2100.000	2486.452	*****	3449.098
Header Type: XS	66.915	*****	.845	93846.56	*****	4149.059
SRD: 4920.000	63.430	*****	.091	*****	1.324	*****
Section: FULL	66.942	.015	2100.000	2487.628	80.000	3449.098
Header Type: FV	66.956	.040	.844	93915.03	80.000	4149.059
SRD: 5000.000	63.470	.000	.091	.0005	1.324	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	66.994	.015	2100.000	2488.959	100.000	3449.098
Header Type: AS	67.008	.050	.844	93992.63	100.000	4149.059
SRD: 5100.000	63.520	.000	.091	.0005	1.324	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 66.94 65.26

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.260	.415	2098.896	406.146	80.000	3760.000
Header Type: BR	65.675	*****	5.168	13092.69	*****	3840.000
SRD: 5000.000	62.450	*****	.572	*****	1.000	*****

Bridge Summary Information - Component Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 3 - Sloping embankments & sloping spillthrough abutments

C	PFELEV	BLN	XLAB	XRAB
.8000	65.260	80.000	3780.795	3819.205

No Pier(s)/Pile(s) Present at Bridge

```
-----
-
Unconstricted Full Valley Section Water Surface Elevation:      66.942
Downstream Bridge Section Water Surface Elevation:              65.260
Bridge DrawDown Distance:                                       1.682
-----
-
```

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	67.864	.009	2100.000	3098.288	80.000	3449.057
Header Type: AS	67.873	.283	.678	132300.80	111.239	4149.076
SRD: 5100.000	63.520	.000	.063	.0005	1.228	-.001

** Change in Approach Section Water Surface Elevation: .870 **

```

Approach Section APPR Flow Contraction Information
M( G )  M( K )      KQ      XLKQ      XRKQ      OTEL
-----
*****  *****  *****  *****  *****  67.864
-----

```

<<< End of Bridge Hydraulics Computations >>>

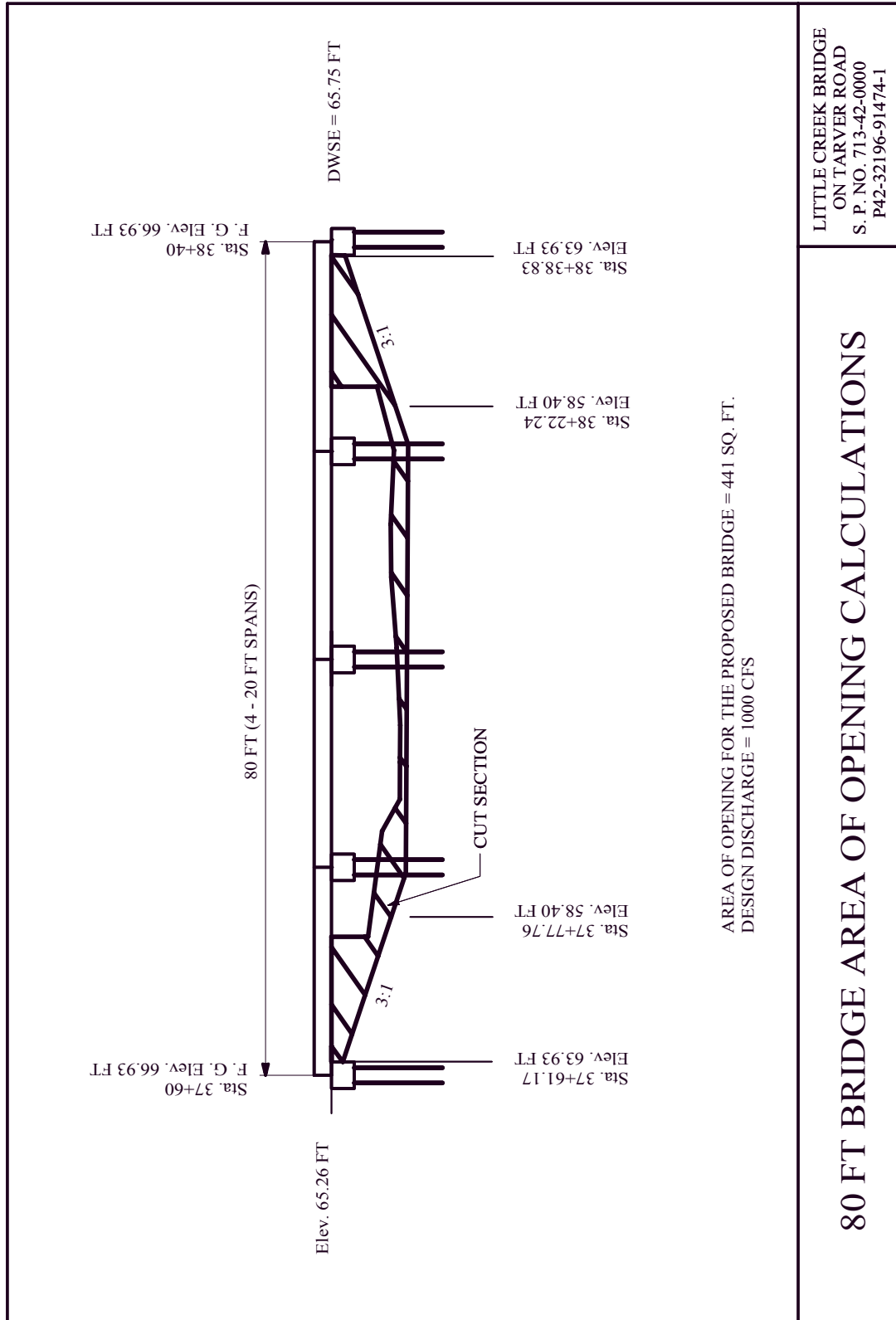
<< Completed Computations of Profile 2 >>

ER

```

***** Normal end of WSPRO execution. *****
***** Elapsed Time:  0 Minutes  1 Seconds *****

```



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**WSPRO Calculations
for the
Proposed Precast 3 Sided Structure
2 – 28' x 7'**

```

*
T1      S.P. No. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      PROPOSED P3S STRUCTURE, 2 - 28' SPAN, 7' RISE (167 sf eq area)
*
*      Q5      Q100
Q      1000    2100
WS      65.495  65.495
*      (water surface elevation from existing approach unconstricted)
*
CV  BOXCV 5000  3800  50  58.40  58.45  2
*      (slope for culvert rounded up to 0.001)
CG      112  72.6  331.2
*      (the height and span set to 6.05 ft and 27.6 ft.)
*      (this gives the same area of opening for the arch box)
*
EX
ER

```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time: 4/23/2005 4:38 pm Version V200104
Input File: SR-P3S28 Output File: SR-P3S28.LST
*-----*
T1      S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      PROPOSED P3S STRUCTURE, 2 - 28' SPAN, 7' RISE (167 SF EQ AREA)
Q        1000      2100

*** Processing Flow Data; Placing Information into Sequence 1 ***

WS      65.495    65.495
```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED P3S STRUCTURE, 2 - 28'SPAN, 7'RISE (167 SF EQ AREA)

*-----*
* Starting To Process Header Record BOXCV *
*-----*

CV BOXCV 5000 3800 50 58.40 58.45 2
CG 112 72.6 331.2

*** Completed Reading Data Associated With Header Record BOXCV ***
*** Notice - Program Assuming All Culvert Input Complete ***
*** Storing Culvert Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record BOXCV ***
SRD Location: 5000. Culvert Code: 112 Error Code 0

Culvert Information:

Shape: Box Material: Concrete
Inlet Code: 2 Length: 50.000
Rise: 72.600 Span: 331.200 # Barrel(s): 2
Roughness: .0120 Horizontal Stationing: 3800.000
Entrance Loss Coefficient: .50 Alpha: 1.0000
Invert Elevations -> Upstream: 58.450
Downstream: 58.400

*-----*
* Finished Processing Header Record BOXCV *
*-----*
```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED P3S STRUCTURE, 2 - 28' SPAN, 7' RISE (167 SF EQ AREA)

EX

=====

* Summary of Boundary Condition Information *

=====

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	1000.00	65.495	*****	Sub-Critical
2	2100.00	65.495	*****	Sub-Critical

=====

* Beginning 2 Profile Calculation(s) *

=====

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED P3S STRUCTURE, 2 - 28'SPAN, 7'RISE (167 SF EQ AREA)
```

```
<< Beginning Computations for Profile 1 >>
WSEL      TWD      Q/BL      AIN      OTFULL
HWic      Dc      VELIN      AOUT      Ac
HWoc      Dn      VELOUT     CVSLPE     An
-----
Section: BOXCV      65.713  7.095   500.000   166.980   1.004
Header Type: CV      3.694  2.167    2.994   166.980   59.819
SRD: *****      7.263  2.692    2.994    .0010   74.299
-----
<< Completed Computations of Profile 1 >>
```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
```

```
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
PROPOSED P3S STRUCTURE, 2 - 28'SPAN, 7'RISE (167 SF EQ AREA)
```

```
<< Beginning Computations for Profile 2 >>
WSEL      TWD      Q/BL      AIN      OTFULL
HWic      Dc      VELIN      AOUT      Ac
HWoc      Dn      VELOUT     CVSLPE     An
-----
Section: BOXCV      66.456  7.095  1050.000   166.980   1.033
Header Type: CV      6.031  3.555    6.288   166.980   98.131
SRD: *****      8.006  4.367    6.288    .0010  120.519
-----
<< Completed Computations of Profile 2 >>
```

ER

```
***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 1 Seconds *****
```

Culvert Calculations
(HYDR1120 Output)

LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
HYDRAULICS SECTION

HYDR1120-071498

DESIGNER: Name

DATE: 04-19-2005

REMARKS : Richland Parish - Little Creek Bridge over Tarver Road

STATE PROJECT NUMBER 713-42-0000

REINFORCED CONCRETE PIPE ARCH (INLET TYPE: 0-PROJECTING)

STATION	Lit_Creek
NUMBER OF PIPES	4
ROUND EQUIVALENT DIAMETER (IN.)	96
DESIGN DISCHARGE (CFS)	1000.00
TAILWATER (FT.)	8.02
LENGTH (FT.)	50.00
SLOPE (FT./FT.)	.00100

HEADWATER (OUTLET)	8.44	FT.
OUTLET VELOCITY	4.85	F.P.S.
DEPTH OF SCOUR FOR TYPE A SOIL	2.70	FT.

$\Delta H = 0.42 \text{ ft}$

LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
HYDRAULICS SECTION

HYDR1120-071498

DESIGNER: Name

DATE: 04-19-2005

REMARKS : Richland Parish - Little Creek Bridge over Tarver Road

STATE PROJECT NUMBER 713-42-0000

CORRUGATED METAL PIPE ARCH (INLET CODE = 0)

STATION	Lit_Creek
NUMBER OF PIPES	4
ROUND EQUIVALENT DIAMETER (IN.)	108
CORRUGATION	3" X 1"
DESIGN DISCHARGE (CFS)	1000.00
TAILWATER (FT.)	8.02
LENGTH (FT.)	50.00
SLOPE (FT./FT.)	.00100

HEADWATER (OUTLET)	8.58	FT.
OUTLET VELOCITY	4.13	F.P.S.
DEPTH OF SCOUR FOR TYPE A SOIL	2.51	FT.

$\Delta H = 0.56 \text{ ft}$

LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
HYDRAULICS SECTION

HYDR1120-071498

DESIGNER: Name

DATE: 04-19-2005

REMARKS : Richland Parish - Little Creek Bridge over Tarver Road

STATE PROJECT NUMBER 713-42-0000

REINFORCED CONCRETE BOX (SQUARE-EDGE STRAIGHT HEADWALLS)

STATION	Lit_Creek
NUMBER OF BOXES	4
SPAN (FEET)	8
HEIGHT (FEET)	7
DESIGN DISCHARGE (CFS)	1000.00
TAILWATER (FT.)	8.02
LENGTH (FT.)	50.00
SLOPE (FT./FT.)	.00100

HEADWATER (OUTLET)	8.46	FT.
OUTLET VELOCITY	4.46	F.P.S.
DEPTH OF SCOUR FOR TYPE A SOIL	2.62	FT.

$\Delta H = 0.44\text{ft}$

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C. Recommendation

After careful consideration of the different alternates, the following structural alternates are recommended for replacing the existing structure over Little Creek on Tarver Road:

One 80 ft (4 – 20 ft spans) Long Reinforced Concrete Bridge
with 3:1 Spill through Abutments
Proposed Finished Grade at Stream Crossing: 66.93 ft

or

2 – 28' x 7'
Precast 3 Sided Structure
Proposed Finished Grade at Stream Crossing: 66.93 ft

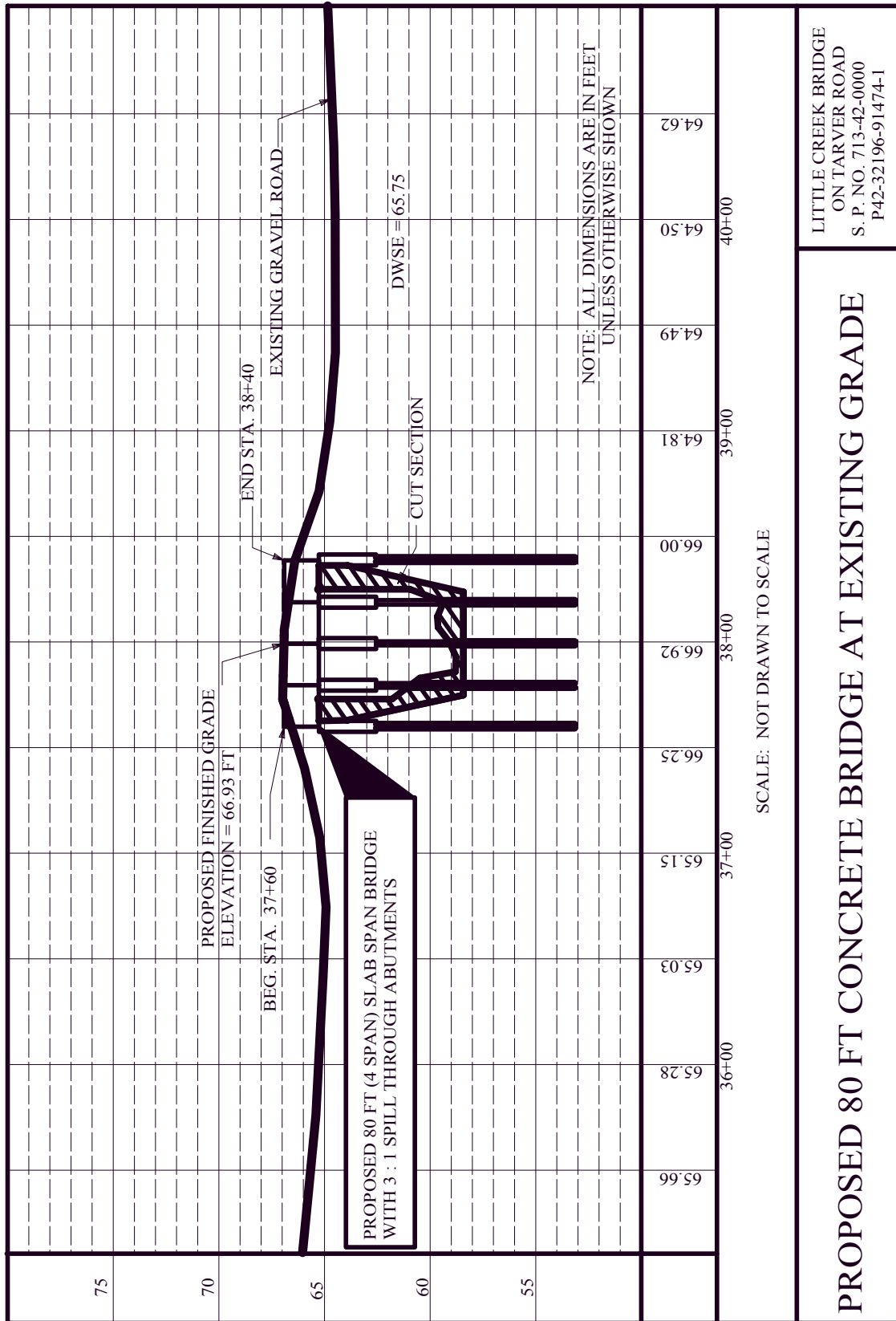
Considering the project feasibility, area of opening, effective width of the creek, and backwater created by the proposed structure, and the design discharge of 1000 cfs for the 5-year frequency, the four span, 80 ft long concrete bridge at the existing grade is recommended. The 60 ft long bridge is not recommended because of the amount of fill that would be required. A table comparison of the hydraulic performance for each alternate and a Hydraulic Data Table for the preferred alternate follows. A profile view for each alternate is also included.

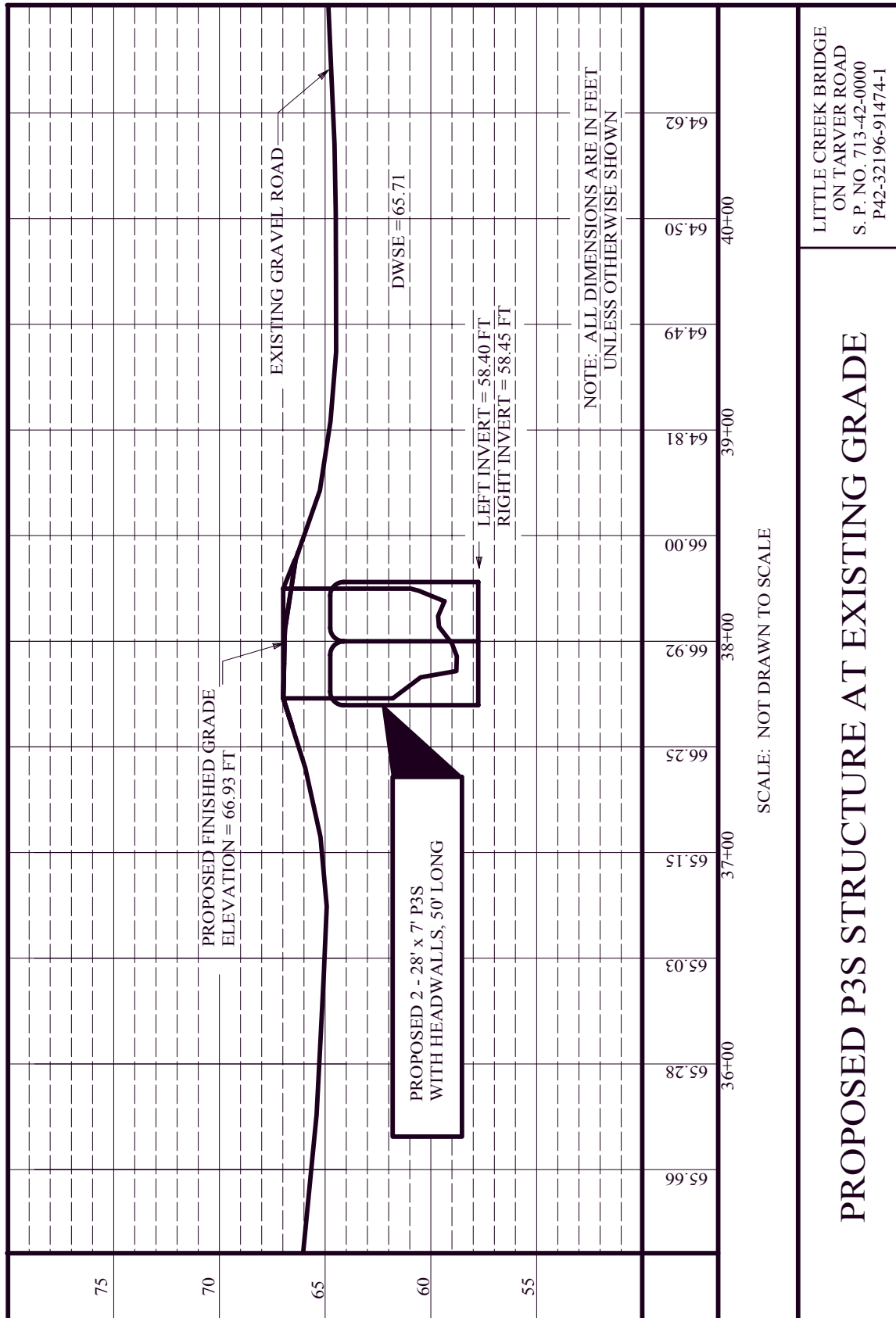
Off-System Bridge Rehabilitation & Replacement Program
COMPARISON TABLE OF STRUCTURAL ALTERNATES TO EXISTING STRUCTURE

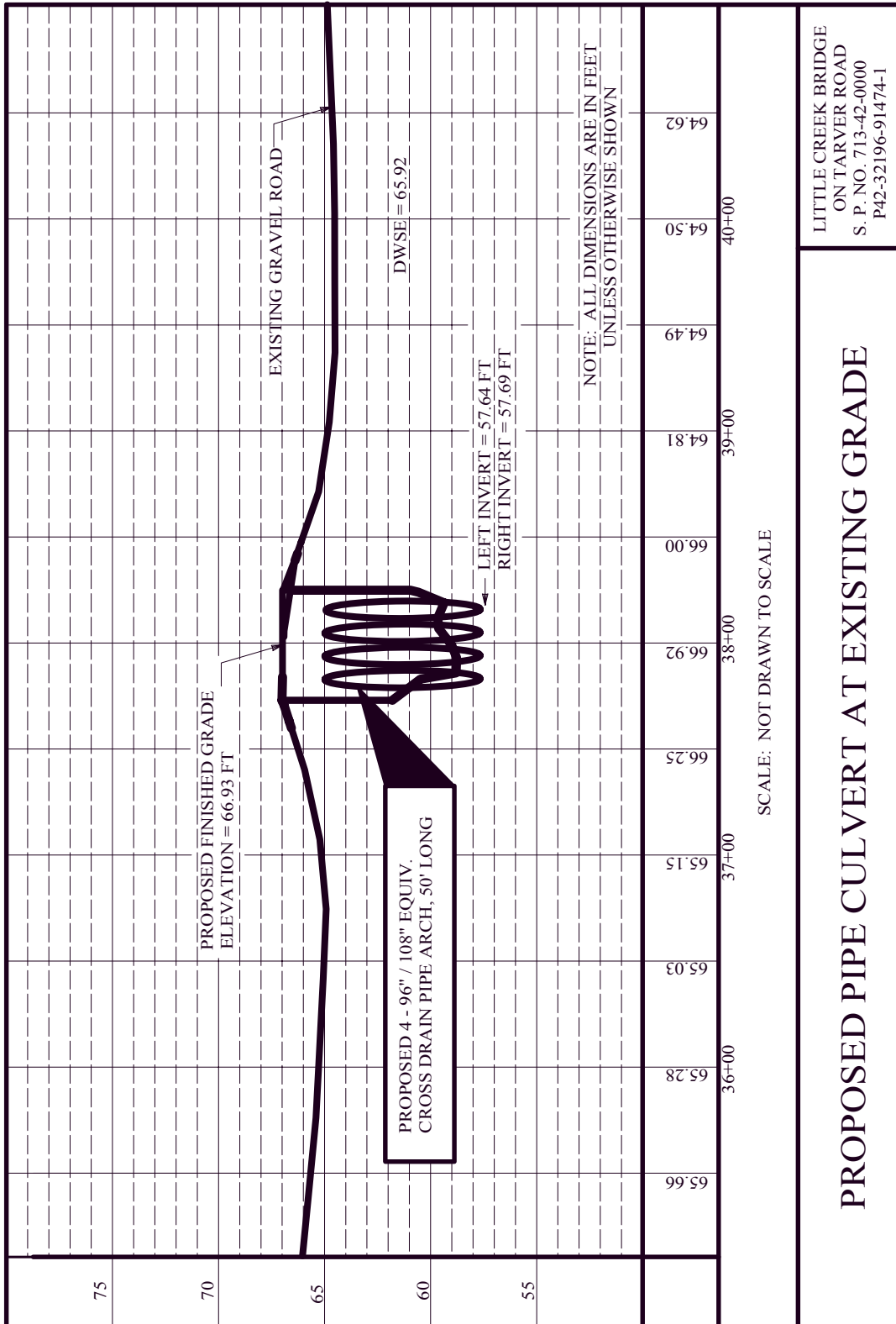
	EXISTING STRUCT.	BRIDGE ALT.	P3S ALT.	RCP / PP ALT.	CMP ALT.	RCB ALT.
SIZE, TYPE & NO.	52' Timber Bridge	4-span, 80' slab span bridge	2 - 28' x 7'	4 - 96" equiv.	4 - 108" equiv.	4 - 8' x 7'
DESIGN FREQUENCY	5 year	5 year	5 year	5 year	5 year	5 year
DESIGN DISCHARGE (cfs)	1000	1000	1000	1000	1000	1000
FINISHED GRADE ELEVATION (ft)	66.93	66.93	66.93	66.93	66.93	66.93
LOW ROADWAY ELEVATION (ft)	64.48	64.48	64.48	64.48	64.48	64.48
DESIGN WATER SURFACE ELEVATION * (ft)	65.93	65.75	65.71	65.92	66.06	65.94
OUTLET VELOCITY (fps)	2.48	2.46	2.99	4.85	4.13	4.46
DEPTH OF SCOUR (ft)						
SOIL TYPE (Culverts Only)				1.62 Soil Type C	1.51 Soil Type C	1.57 Soil Type C
AREA OF OPENING (ft²)	300	406	334	201	255	224
DIFFERENTIAL HEAD (ft)	0.439	0.243	0.21	0.42	0.56	0.44

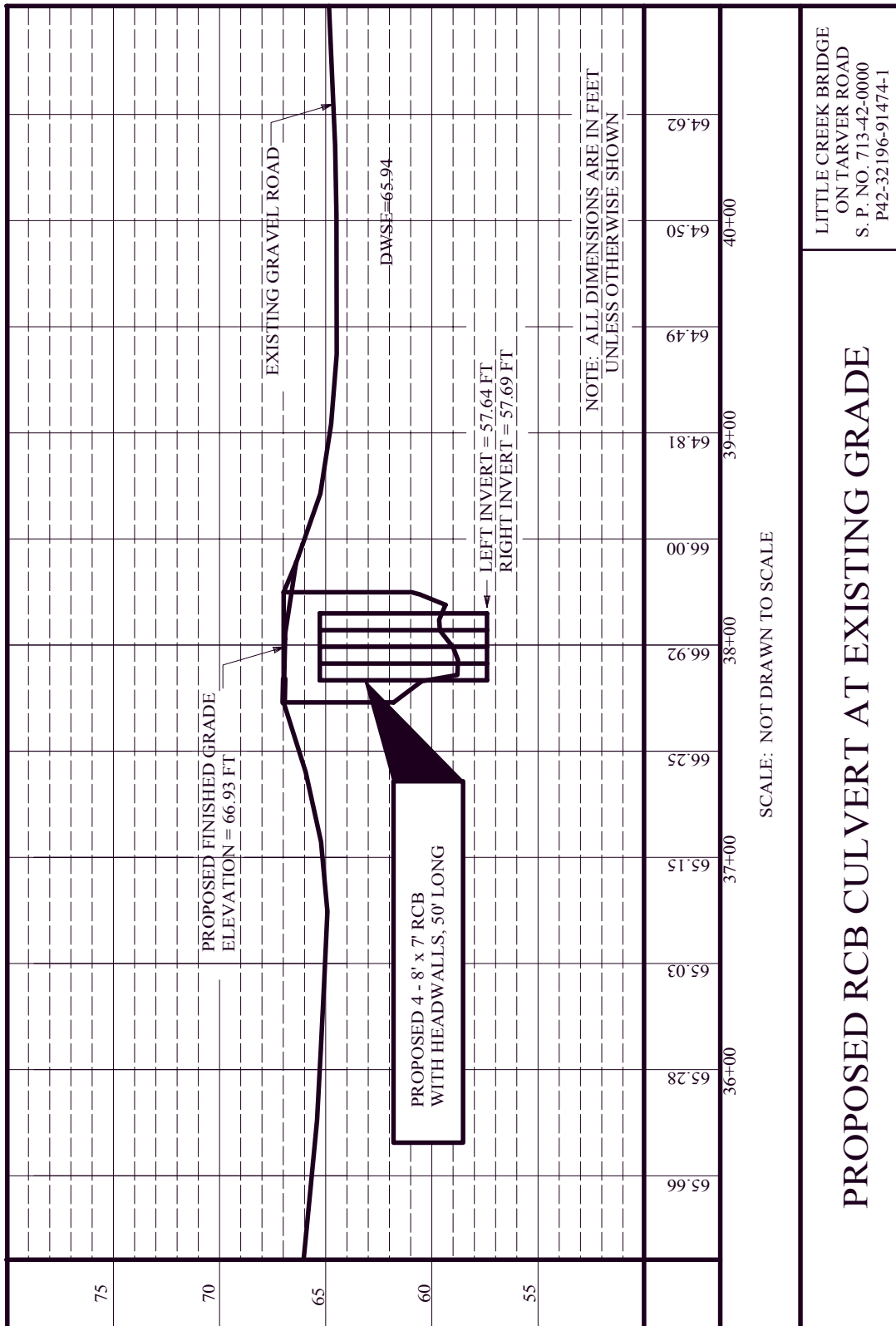
* Includes the differential head, does not reflect roadway overtopping

2005









Off-System Bridge Rehabilitation & Replacement Program Bridge Hydraulic Data Form

STATE PROJECT NO.: 713-42-0000

STREAM NAME: LITTLE CREEK

PARISH: RICHLAND

BEGINNING STATION: 37+60

STRUCTURE NO.: P42-32196-91474-1

FINISH GRADE ELEV. (ft): 66.93

PREPARED BY: J. K.

BRIDGE LENGTH (ft): 80

DATE: 2005

LOW ROADWAY ELEVATION (ft): 64.48

NOTE: THE FOLLOWING HYDRAULIC DATA TABLE IS TO BE PLACED ON THE PLANS.

HYDRAULIC DATA TABLE

Drainage Area (mi ²): 19.64				PREDICTED SCOUR	
Basin Slope (ft/mi): 3.02					
Flood Frequency (years)	Design Year (5)	Design Year (5)	100	Flood Frequency (years)	
Discharge (ft ³ /s)	1000	1000	2100	Discharge (ft ³ /s)	
Structure	Existing Bridge	Proposed Structure	Proposed Structure	Contraction Scour Depth (ft)	
Size & Type	52 ft bridge	80 ft bridge	80 ft bridge		
Design Water Surface Elev. (ft/MSL)	65.93*	65.75*	67.86*	Maximum Local Scour Depth (ft)	
Average Velocity (ft/s)	2.48	2.46	5.17		
Area of Opening (ft ²)	300	406	406	Abutment Scour Depth (ft)	
Backwater (ft)	0.439	0.243	0.870	Bridge Scour Elevation (ft)	

REMARKS: * Includes backwater; does not reflect roadway overtopping

SCOUR: To be calculated after the Plan-in-Hand

EXISTING BRIDGE: 4-span timber bridge, 52 feet total length

DETOUR BRIDGE: not required

2005

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SAMPLE

SCOUR SUBMITTAL

The following is a sample scour submittal for the sample hydraulic report. The intent of providing this is for general format only. **This sample is not for setting policy. (For policy see the first portion of the guidelines.)** The methods, assumptions, and design criteria used in the calculations were considered appropriate for this particular bridge site. Each bridge site is unique and variations in the WSPRO model and scour analyses may be appropriate and acceptable as long as they follow sound hydraulic reasoning.

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I. SCOUR ANALYSIS

The plan-in-hand party has decided that the bridge alternate will be the replacement structure. In view of this, the scour has been determined for the proposed 80 ft (4-span) bridge according to “HEC 18 – Evaluating Scour, fourth edition” (May 2001). The calculations and an updated Hydraulic Data Table follow the WSPRO run.

Since the water overtops the roadway at a frequency less than 5 years, and based on HEC-18, it was decided that the scour should be determined for the 5-year frequency and not the 100-year or 500-year frequency. For the 5-year discharge, clear-water was the governing factor for the contraction scour.

It is recommended that flexible revetment be placed on the abutments; therefore, the calculation of abutment scour was not necessary.

The contraction scour was determined to be 9.72 ft and the pier scour 2.58 ft. The total scour estimation based on the calculations is 12.30 ft. There was not any evidence of scour problems at the site, and the discharge and velocity are low. It is felt that the 12.30 ft of scour is too high an estimation; therefore, it is recommended that the piles be designed for the minimum total scour depth of 5.0 ft.

```

*
T1      S.P. No. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE
*
*      Q5
Q      1000
SK      .0005
*
XT      TEMP 5000
GR      3449,69.00  3449,66.90  3508,65.29
GR      3606,63.09  3655,64.14  3693,64.17
GR      3734,62.04  3767,61.41  3799,59.19
GR      3831,60.92  3836,62.20  3852,64.14
GR      3856,64.27  3954,62.34  4053,63.71
GR      4149,64.04  4149,69.00
*
XS      EXIT 4920  *  *  *  .0005
GT
N      0.100      0.085      0.100
SA      3767      3831
*
XS      FULL 5000  *  *  *  .0005
*
BR      BRDG 5000  65.26  *  *  .0005
GR      3761.17,63.93  3777.76,58.40
GR      3822.24,58.40  3838.83,63.93
BC      65.26
BL      80      3800      3800
N      0.085
CD      3  20.00  3  66.93
AB      3.0
*
*      XR      ROAD 5000  20.00  1
*      GR      3445,68.02  3478,66.36  3543,65.72  3609,65.23
*      GR      3674,64.92  3707,65.22  3740,65.94  3773,66.98
*      GR      3806,66.90  3839,66.40  3871,65.25  3904,64.75
*      GR      3937,64.48  4035,64.56  4101,64.85  4134,64.95
*
XS      APPR 5100  *  *  *  .0005
*
HP 2 BRDG 65.250  1  65.250  1000
HP 1 BRDG 65.250  1  65.250
*
HP 1 APPR 65.753  1  65.753
*
EX
ER

```

```
***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Run Date & Time:  9/27/2005  5:05 pm      Version V200104
Input File: SA-PBR-E      Output File: SA-PBR-E.LST
*-----*
T1      S.P. NO. 713-42-0000  RICHLAND PARISH LITTLE CREEK BRIDGE
T2      STR. NO. P42-32196-91474-1
T3      SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE
Q      1000

***      Processing Flow Data; Placing Information into Sequence  1      ***

SK      .0005
```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

* Starting To Process Header Record TEMP *

XT	TEMP	5000			
GR		3449,69.00	3449,66.90	3508,65.29	
GR		3606,63.09	3655,64.14	3693,64.17	
GR		3734,62.04	3767,61.41	3799,59.19	
GR		3831,60.92	3836,62.20	3852,64.14	
GR		3856,64.27	3954,62.34	4053,63.71	
GR		4149,64.04	4149,69.00		

*** Completed Reading Data Associated With Header Record TEMP ***
*** Storing Template Header Record Data In Memory ***

*** Data Summary For Header Record TEMP ***
SRD Location: 5000. Valley Slope: ***** Error Code 0

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.000	3449.100	66.900	3508.000	65.290
3606.000	63.090	3655.000	64.140	3693.000	64.170
3734.000	62.040	3767.000	61.410	3799.000	59.190
3831.000	60.920	3836.000	62.200	3852.000	64.140
3856.000	64.270	3954.000	62.340	4053.000	63.710
4149.000	64.040	4149.100	69.000		

+++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment.
+++072 NOTICE: X-coordinate #17 increased to eliminate vertical segment.

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.000)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.000)
Minimum Y-Elevation: 59.190 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.000 (associated X-Station: 3449.000)

* Finished Processing Header Record TEMP *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

* Starting To Process Header Record EXIT *

XS EXIT 4920 * * * .0005
GT
N 0.100 0.085 0.100
SA 3767 3831

*** Completed Reading Data Associated With Header Record EXIT ***
*** Storing X-Section Data In Temporary File As Record Number 1 ***

*** Data Summary For Header Record EXIT ***
SRD Location: 4920. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	68.960	3449.100	66.860	3508.000	65.250
3606.000	63.050	3655.000	64.100	3693.000	64.130
3734.000	62.000	3767.000	61.370	3799.000	59.150
3831.000	60.880	3836.000	62.160	3852.000	64.100
3856.000	64.230	3954.000	62.300	4053.000	63.670
4149.000	64.000	4149.100	68.960		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 68.960)
Maximum X-Station: 4149.100 (associated Y-Elevation: 68.960)
Minimum Y-Elevation: 59.150 (associated X-Station: 3799.000)
Maximum Y-Elevation: 68.960 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
		3767.000
2	.085	---
		3831.000
3	.100	---

* Finished Processing Header Record EXIT *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

* Starting To Process Header Record FULL *

XS FULL 5000 * * * .0005

*** Completed Reading Data Associated With Header Record FULL ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 2 ***

*** Data Summary For Header Record FULL ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.000	3449.100	66.900	3508.000	65.290
3606.000	63.090	3655.000	64.140	3693.000	64.170
3734.000	62.040	3767.000	61.410	3799.000	59.190
3831.000	60.920	3836.000	62.200	3852.000	64.140
3856.000	64.270	3954.000	62.340	4053.000	63.710
4149.000	64.040	4149.100	69.000		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.000)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.000)
Minimum Y-Elevation: 59.190 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.000 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	3767.000
2	.085	---
	---	3831.000
3	.100	---

* Finished Processing Header Record FULL *

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

* Starting To Process Header Record BRDG *

BR BRDG 5000 65.26 * * .0005
GR 3761.17, 63.93 3777.76, 58.40
GR 3822.24, 58.40 3838.83, 63.93
BC 65.26
BL 80 3800 3800
N 0.085
CD 3 20.00 3 66.93
AB 3.0

*** Completed Reading Data Associated With Header Record BRDG ***
*** Storing Bridge Data In Temporary File As Record Number 3 ***

*** Data Summary For Bridge Record BRDG ***
SRD Location: 5000. Cross-Section Skew: .0 Error Code 0
Valley Slope: ***** Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (5 pairs)					
X	Y	X	Y	X	Y
3760.000	65.260	3780.795	58.400	3819.205	58.400
3840.000	65.260	3760.000	65.260		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3760.000 (associated Y-Elevation: 65.260)
Maximum X-Station: 3840.000 (associated Y-Elevation: 65.260)
Minimum Y-Elevation: 58.400 (associated X-Station: 3819.205)
Maximum Y-Elevation: 65.260 (associated X-Station: 3760.000)

Roughness Data (1 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.085	---

Discharge coefficient parameters				
BRTYPE	BRWidth	EMBSS	EMBElv	UserCD
3	20.000	3.00	66.930	*****

Pressure flow elevations	
AVBCEL	PFElev
65.260	65.260

Abutment Parameters					
ABSLPL	ABSLPR	XTOELT	YTOELT	XTOERT	YTOERT
3.000	3.000	3780.795	58.400	3819.205	58.400

Bridge Length and Bottom Chord component input data						
BRLEN	LOCOPT	XCONLT	XCONRT	BCELEV	BCSLP	BCXSTA
80.000	0	3800.000	3800.000	65.260	.0000	3800.000

** No Pier/Pile Data Encountered **

```
*-----*
*      Finished Processing Header Record BRDG      *
*-----*
```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

* Starting To Process Header Record APPR *

XS APPR 5100 * * * .0005

*** Completed Reading Data Associated With Header Record APPR ***
*** No Roughness Data Input, Propagating From Previous Section ***
*** Storing X-Section Data In Temporary File As Record Number 4 ***

*** Data Summary For Header Record APPR ***
SRD Location: 5100. Cross-Section Skew: .0 Error Code 0
Valley Slope: .00050 Averaging Conveyance By Geometric Mean.
Energy Loss Coefficients -> Expansion: .50 Contraction: .00

X,Y-coordinates (17 pairs)					
X	Y	X	Y	X	Y
3449.000	69.050	3449.100	66.950	3508.000	65.340
3606.000	63.140	3655.000	64.190	3693.000	64.220
3734.000	62.090	3767.000	61.460	3799.000	59.240
3831.000	60.970	3836.000	62.250	3852.000	64.190
3856.000	64.320	3954.000	62.390	4053.000	63.760
4149.000	64.090	4149.100	69.050		

Minimum and Maximum X,Y-coordinates
Minimum X-Station: 3449.000 (associated Y-Elevation: 69.050)
Maximum X-Station: 4149.100 (associated Y-Elevation: 69.050)
Minimum Y-Elevation: 59.240 (associated X-Station: 3799.000)
Maximum Y-Elevation: 69.050 (associated X-Station: 3449.000)

Roughness Data (3 SubAreas)		
SubArea	Roughness Coefficient	Horizontal Breakpoint
1	.100	---
	---	3767.000
2	.085	---
	---	3831.000
3	.100	---

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

* Finished Processing Header Record APPR *

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English
*-----*
S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE
HP 2 BRDG 65.250 1 65.250 1000
HP 1 BRDG 65.250 1 65.250
HP 1 APPR 65.753 1 65.753
EX

```

```

*=====*
* Summary of Boundary Condition Information *
*=====*

```

#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
1	1000.00	*****	.0005	Sub-Critical

```

*=====*
* Beginning 1 Profile Calculation(s) *
*=====*

```

***** W S P R O *****
Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

<< Beginning Computations for Profile 1 >>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: EXIT	65.416	.011	1000.000	1485.965	*****	3501.922
Header Type: XS	65.427	*****	.673	44691.34	*****	4149.029
SRD: 4920.000	62.181	*****	.099	*****	1.603	*****
Section: FULL	65.458	.011	1000.000	1487.107	80.000	3501.858
Header Type: FV	65.469	.040	.672	44739.42	80.000	4149.029
SRD: 5000.000	62.221	.000	.099	.0005	1.602	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: APPR	65.510	.011	1000.000	1488.451	100.000	3501.782
Header Type: AS	65.521	.050	.672	44796.08	100.000	4149.029
SRD: 5100.000	62.271	.000	.099	.0005	1.602	.002

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>>

<<< Beginning Bridge/Culvert Hydraulic Computations >>>

===255 ATTEMPTING FLOW CLASS 3 OR 6 SOLUTION.

WS3N, PFelv: 65.46 65.26

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDG	65.260	.096	1008.243	406.146	80.000	3760.000
Header Type: BR	65.356	*****	2.482	13092.69	*****	3840.000
SRD: 5000.000	60.984	*****	.275	*****	1.000	*****

Bridge Summary Information - Component Mode

Flow Class: 3 - Submerged orifice flow with no embankment overtopping
Bridge Type: 3 - Sloping embankments & sloping spillthrough abutments

C	PFELEV	BLN	XLAB	XRAB
.8000	65.260	80.000	3780.795	3819.205

No Pier(s)/Pile(s) Present at Bridge

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Unconstricted Full Valley Section Water Surface Elevation: 65.458
Downstream Bridge Section Water Surface Elevation:      65.260
Bridge DrawDown Distance:                               .198
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	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: APPR	65.753	.009	1000.000	1646.539	80.000	3492.907
Header Type: AS	65.761	.154	.607	51651.63	104.099	4149.034
SRD: 5100.000	62.271	.000	.084	.0005	1.532	.008

** Change in Approach Section Water Surface Elevation: .243 **

Approach Section APPR Flow Contraction Information					
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	65.753

<<< End of Bridge Hydraulics Computations >>>

<< Completed Computations of Profile 1 >>

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SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

*** Beginning Velocity Distribution For Header Record BRDG ***
SRD Location: 5000.000 Header Record Number 3

Water Surface Elevation: 65.250 Element # 1
Flow: 1000.000 Velocity: 2.47 Hydraulic Depth: 5.071
Cross-Section Area: 405.35 Conveyance: 20540.26
Bank Stations -> Left: 3760.030 Right: 3839.970

X STA.	3760.0	3774.7	3778.9	3782.0	3784.7	3787.4
A(I)		35.5	22.9	20.9	18.3	18.5
V(I)		1.41	2.18	2.39	2.73	2.71
D(I)		2.42	5.53	6.65	6.85	6.85

X STA.	3787.4	3789.9	3792.5	3795.0	3797.5	3800.0
A(I)		17.6	17.5	17.3	17.1	17.1
V(I)		2.84	2.85	2.89	2.93	2.93
D(I)		6.85	6.85	6.85	6.85	6.85

X STA.	3800.0	3802.5	3805.0	3807.5	3810.1	3812.6
A(I)		17.1	17.1	17.3	17.5	17.6
V(I)		2.93	2.93	2.89	2.85	2.84
D(I)		6.85	6.85	6.85	6.85	6.85

X STA.	3812.6	3815.3	3818.0	3821.1	3825.3	3840.0
A(I)		18.5	18.3	20.9	22.9	35.5
V(I)		2.71	2.73	2.39	2.18	1.41
D(I)		6.85	6.85	6.65	5.53	2.42

Water Surface Elevation: 65.260 Element # 2
Flow: 1000.000 Velocity: 2.46 Hydraulic Depth: 5.077
Cross-Section Area: 406.15 Conveyance: 13092.69
Bank Stations -> Left: 3760.000 Right: 3840.000

X STA.	3760.0	3774.5	3778.8	3781.9	3784.6	3787.3
A(I)		34.8	23.5	20.4	18.9	18.1
V(I)		1.44	2.13	2.46	2.64	2.76
D(I)		2.40	5.50	6.65	6.86	6.86

X STA.	3787.3	3789.9	3792.4	3795.0	3797.5	3800.0
A(I)		17.7	17.7	17.4	17.2	17.2
V(I)		2.82	2.83	2.87	2.90	2.90
D(I)		6.86	6.86	6.86	6.86	6.86

X STA.	3800.0	3802.5	3805.0	3807.6	3810.1	3812.7
A(I)		17.2	17.2	17.4	17.7	17.7
V(I)		2.90	2.90	2.87	2.83	2.82
D(I)		6.86	6.86	6.86	6.86	6.86

X STA.	3812.7	3815.4	3818.1	3821.2	3825.5	3840.0
A(I)		18.1	18.9	20.4	23.5	34.8
V(I)		2.76	2.64	2.46	2.13	1.44
D(I)		6.86	6.86	6.65	5.50	2.40

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Federal Highway Administration - U. S. Geological Survey
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STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

*** Compute Cross-Section Properties For Header Record BRDG ***
SRD Location: 5000.000 Header Record Number 3

Water Surface Elevation	S A #	Cross Section Conveyance	Cross Section Area(s)	Top Width	Wetted Pmtr	Bank Station ----- Left Right		Hydrlic Depth	Critical Flow
65.250	1	20540.26	405.	79.9	82.14			5.071	5179.50
		20540.26	405.	79.9	82.14	3760.0	3840.0	5.071	5179.50
Velocity Head Correction Factor (alpha): 1.000									
65.260	1	13092.69	406.	160.0	162.20			2.538	3671.91
		13092.69	406.	160.0	162.20	3760.0	3840.0	2.538	3671.91
Velocity Head Correction Factor (alpha): 1.000									

***** W S P R O *****

Federal Highway Administration - U. S. Geological Survey
Model for Water-Surface Profile Computations.
Input Units: English / Output Units: English

S.P. NO. 713-42-0000 RICHLAND PARISH LITTLE CREEK BRIDGE
STR. NO. P42-32196-91474-1
SCOUR CALCULATIONS FOR PROPOSED 4-SPAN, 80 FT SLAB SPAN BRIDGE

*** Compute Cross-Section Properties For Header Record APPR ***
SRD Location: 5100.000 Header Record Number 4

Water Surface Elevation	S A #	Cross Section Conveyance	Cross Section Area(s)	Top Width	Wetted Pmtr	Bank Station ----- Left Right		Hydrlic Depth	Critical Flow
65.753	1	13011.37	550.	274.1	274.21			2.008	4424.78
	2	19297.76	354.	64.0	64.12			5.525	4716.99
	3	19355.45	743.	318.0	319.97			2.336	6442.66
		51664.58	1647.	656.1	658.31	3492.9	4149.0	2.510	11962.73
		Velocity Head Correction Factor (alpha): 1.532							
66.753	1	24349.89	843.	310.7	310.81			2.712	7875.70
	2	25463.06	418.	64.0	64.12			6.525	6053.81
	3	34981.97	1061.	318.1	320.97			3.336	10995.17
		84794.92	2321.	692.7	695.91	3456.3	4149.1	3.351	20734.21
		Velocity Head Correction Factor (alpha): 1.352							
67.753	1	40772.79	1160.	317.9	318.82			3.648	12571.96
	2	32293.36	482.	64.0	64.12			7.525	7497.40
	3	54042.68	1379.	318.1	321.97			4.335	16293.04
		127108.80	3021.	700.0	704.92	3449.1	4149.1	4.315	32005.15
		Velocity Head Correction Factor (alpha): 1.238							
68.753	1	60927.72	1478.	318.0	319.82			4.648	18079.54
	2	39757.65	546.	64.0	64.12			8.525	9040.40
	3	76220.74	1697.	318.1	322.97			5.335	22243.29
		176906.10	3721.	700.1	706.92	3449.0	4149.1	5.315	44974.84
		Velocity Head Correction Factor (alpha): 1.171							
69.050	1	67512.50	1572.	318.0	320.12			4.944	19839.57
	2	42092.77	565.	64.0	64.12			8.823	9516.90
	3	83372.13	1792.	318.1	323.27			5.632	24126.13
		192977.40	3929.	700.1	707.52	3449.0	4149.1	5.611	49084.92
		Velocity Head Correction Factor (alpha): 1.157							

ER

***** Normal end of WSPRO execution. *****
***** Elapsed Time: 0 Minutes 1 Seconds *****

Scour Calculations

CONTRACTION SCOUR

Determine if it is live-bed or clear-water scour.

$$V_c = 11.17 y^{1/6} D_{50}^{1/3}$$

Where :

$$y = \text{depth of flow} = \frac{A_{\text{ch-appr}}}{\text{Topw}_{\text{appr}}} = \frac{354}{64} = 5.53 \text{ ft}$$

$$D_{50} = 0.0002 \text{ m, (assumed a reasonable value)}$$

$$V_c = 11.17 (5.53)^{1/6} (0.0002 \text{ m} * 3.28 \text{ ft/m})^{1/3} = 1.29 \text{ ft/s}$$

The average velocity in the channel equals:

$$V = \frac{Q_1}{A_{\text{ch}}}$$

Where :

$$Q_1 = Q_{\text{total}} \left(\frac{K_1}{K_{\text{total}}} \right) = 1000 \left(\frac{19297.76}{51664.58} \right) = 373.52 \text{ ft}^3/\text{s}$$

$$A_{\text{ch}} = 354$$

$$V = \frac{373.5}{354} = 1.06 \text{ ft/s}$$

$V_c > V$; therefore, the flow condition will be clear-water.

CONTRACTION SCOUR CONTD.

Clear-Water Contraction Scour:

$$y_s = y_2 - y_0 :$$

$$y_2 = \left[\frac{0.0077 (Q)^2}{(D_m)^{2/3} (W_{\text{contracted}})^2} \right]^{3/7}$$

Where :

$$D_m = 1.25 \times D_{50} = 1.25 (0.0002 \text{ m} * 3.28 \text{ ft/m}) = 0.00082 \text{ ft}$$

$$Q = 1000 \text{ ft}^3/\text{s}, \text{ (discharge through the bridge)}$$

$$W = 44.48 - 3 (1.33) = 40.49 \text{ ft}$$

$$y_2 = \left[\frac{0.0077 (1000)^2}{(0.00082)^{2/3} (40.49)^2} \right]^{3/7} = 14.78 \text{ ft}$$

$$y_0 = \frac{A_{BR}}{TOPW_{BR}} = \frac{405}{79.9} = 5.06 \text{ ft}$$

$$y_s = y_2 - y_0$$

$$= 14.78 \text{ ft} - 5.06 \text{ ft}$$

$$y_s = 9.72 \text{ ft}$$

PIER SCOUR

$$\frac{y_s}{y_1} = 2.0 K_1 K_2 K_3 K_4 (a/y_1)^{0.65} F_{r1}^{0.43}$$

Where :

for a round - nose pier, aligned with the flow,
sand - bed material :

$$K_1 = K_2 = K_4 = 1.0$$

for plane - bed condition

$$K_3 = 1.1$$

$$V = 2.93 \text{ ft/s}$$

$$y_1 = 6.85 \text{ ft}$$

$$F_{r1} = \frac{V}{(g y_1)^{0.5}} = \frac{(2.93)}{[(32.2)(6.85)]^{0.5}} = 0.197$$

$$\frac{y_s}{6.85} = 2.0 (1) (1) (1.1) (1) (1.33/6.85)^{0.65} (0.197)^{0.43}$$

$$y_s = 2.58 \text{ ft}$$

TOTAL SCOUR:

$$9.72 \text{ ft} + 2.58 \text{ ft} = 12.30 \text{ ft}$$

Off-System Bridge Rehabilitation & Replacement Program Bridge Hydraulic Data Form

STATE PROJECT NO.: 713-42-0000

STREAM NAME: LITTLE CREEK

PARISH: RICHLAND

BEGINNING STATION: 37+60

STRUCTURE NO.: P42-32196-91474-1

FINISH GRADE ELEV. (ft): 66.93

PREPARED BY: J. K.

BRIDGE LENGTH (ft): 80

DATE: 2005

LOW ROADWAY ELEVATION (ft): 64.48

NOTE: THE FOLLOWING HYDRAULIC DATA TABLE IS TO BE PLACED ON THE PLANS.

HYDRAULIC DATA TABLE

Drainage Area (mi ²): 19.64				PREDICTED SCOUR	
Basin Slope (ft/mi): 3.02					
Flood Frequency (years)	Design Year (5)	Design Year (5)	100	Flood Frequency (years)	5
Discharge (ft ³ /s)	1000	1000	2100	Discharge (ft ³ /s)	1000
Structure	Existing Bridge	Proposed Structure	Proposed Structure	Contraction Scour Depth (ft)	9.72**
Size & Type	52 ft bridge	80 ft bridge	80 ft bridge		
Design Water Surface Elev. (ft/MSL)	65.93*	65.75*	67.86*	Maximum Pier Scour Depth (ft)	2.58**
Average Velocity (ft/s)	2.48	2.46	5.17		
Area of Opening (ft ²)	300	406	406	Abutment Scour Depth (ft)	protected
Backwater (ft)	0.439	0.243	0.870	Bridge Scour Elevation (ft)	53.40

REMARKS: * Includes backwater; does not reflect roadway overtopping
Flexible revetment is recommended to 5 ft outside the fascia of the structure on the upstream and downstream sides

SCOUR: ** Since there is no evidence of past scour at the site and the velocity is low, a minimum total scour depth of 5 ft is recommended.

EXISTING BRIDGE: 4-span timber bridge, 52 feet total length

DETOUR BRIDGE: not required

2005